

U.S. Department of Transportation Federal Highway Administration



LTPP Seasonal Monitoring Program

Site Installation and Initial Data Collection Section 501002, New Haven Vermont

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LTPP Seasonal Monitoring Program

Site Installation and Initial Data Collection Section 501002, New Haven, Vermont

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The report is a cooperative effort between Vermont Agency of Transportation (VAOT) Materials and Research Division, Long Term Pavement Performance (LTPP) Division Federal Highway Administration, and Pavement Management Systems Limited LTPP North Atlantic Region Coordination Office.

16. Abstract

This report provides a description of the installation of seasonal monitoring instrumentation and initial data collection for the seasonal experimental study conducted as part of the Long Term Pavement Performance (LTPP) program at the General Pavement Study (GPS) section 501002 on RT 7 in New Haven, Vermont. This asphalt concrete surface pavement test section was instrumented on October 6, 1993. The instrumentation installed included time domain reflectometry probes for moisture content, electrical resistivity probes for frost location, thermistor probes for temperature, tipping bucket rain gage, piezometer to monitor the ground water table, and an on-site data logger. Initial data collection was performed on October 7, 1993 which consisted of deflection measurements with a Falling Weight Deflectometer, elevation measurements, temperature measurements, TDR measurements, and electrical resistance and resistivity measurements. The report contains a description of the test site and its location, the instruments installed at the site and their locations, characteristics of the installed instruments and probes, problems encountered during installation, specific site circumstances and deviations from the standard guidelines, and a summary of the initial data collection.

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SEASONAL INSTRUMENTATION STUDY INSTRUMENTATION INSTALLATION VERMONT SECTION 501002

I. Introduction

Background (State Experiment)

The state of Vermont in conjunction with the Army Corps of Engineers, Cold Region Research and Engineering Laboratory (CRREL), Hanover, NH, initiated a Seasonal Monitoring Program on four SHRP sites in the fall of 1989. These sites were instrumented in conjunction with the initial FWD testing and material sampling at the SHRP sites. Watermark sensors (moisture) and thermistor sensors (temperature) were installed in the test pit locations at each site. A monitoring well to determine the water table level was also installed at each site. The initial findings were reported by Wendy L. Allen, US Army Corps of Engineers, Cold Region Research and Engineering Laboratory, Hanover, NH (Internal Report 1053, January 1990).

In the spring of 1991, datalogger/storage modules were installed at sites 501002 - New Haven, 501004 - South Hero, and 501681 - Charlotte for continuous data collection from the watermark sensors and thermistor strings. In addition to the SHRP sites, instrumentation was also installed at two locations on route 62 in Berlin and adjacent to the Materials and Research Facility parking lot on town highway 27 in Berlin. In conjunction with the installation of the equipment cabinet and data logger/storage module at site 501002 in New Haven on April 3, 1991, the SHRP FWD collected four sets of data as a trial test procedure for the planned LTPP seasonal data collection. Appendix A provides plots of deflection and temperature variations with time as recorded during FWD testing for 501002 on April 3, 1991 and for town highway 27 on April 4, 1991. Pictures documenting the installation of the data logger/storage module and FWD testing are provided in Appendix E.

In the fall of 1992, new sets of instrumentation were installed at the three SHRP sites and the two sites in Berlin. This instrumentation included:

- MRC thermistor probe (3.05m)
- TDR moisture probes (2 prong)
- Watermark moisture sensors
- Air temperature probe
- Wind speed/direction recorder
- TI tipping bucket rain gage

The installation at site 501004 - South Hero was visited by Aramis Lopez (LTPP) and Brandt Henderson (NARO). The sites instrumented by Vermont Agency of Transportation (VAOT) were to be included in the LTPP seasonal program as supplemental sites. Details outlining the scope of the project and installation plans are provided in Dick Haupt's letter of October 5, 1992 (Appendix B).

Site Specifics (LTPP Experiment)

The installation of the LTPP instrumentation on seasonal site 501002 near New Haven, Vermont was performed on October 6 - October 7, 1993. The test section is a GPS-1 experiment, located on northbound Route 7, approximately 0.16 kilometers south of New Haven, Junction RT 17, 2.08 kilometers north of Middlebury, and 9.6 kilometers west of Bristol city limits (Figure A-1 in Appendix A). The highway consists of one 3.7 m wide lane in each direction with a 2.44 m wide paved outside shoulder.

The pavement structure, which elevates the roadway in a wet low lying plain between two hills, consists of 216 mm of asphalt concrete on 655 mm of crushed gravel base over a coarse grained poorly graded gravel with silt and sand. The depth to rock below road surface is more than 7.5 m. Pavement structure information from the GPS material drilling logs is presented in Appendix A, Figure A-2. Properties determined from the laboratory material tests are shown in Table 1. The materials and layers encountered during the installation differ from what was encountered during the drilling and sampling done in 1989.

Table A-1 in Appendix A summarizes the distress, IRI values from the Profilometer longitudinal profile measurements, and Falling Weight Deflectometer deflection values as monitored since 1989. The uniformity survey results are summarizes in Table A-2 and the deflection values and analysis results from the FWDCHECK are also presented in Appendix A.

The site is in a wet-freeze zone and resides in cell 12 (thick AC on fine subgrade) of the Seasonal Monitoring Program. The annual average frost depth is 1.40 m and the maximum is 1.65 m. Salt is frequently used for ice control at this location. Below is a summary from the LTPP climate database based on seven years of data:

•	Freezing Index (C-Days)	766
•	Precipitation (mm)	1041
•	No. of Freeze/Thaw Cycles	99
•	Days Above 32C	1
•	Days Below 0C	157
•	Wet Days	192

This portion of route 7 was reconstructed and opened to traffic in 1984. The WIM location is on route US 7, 0.6 km South of route 17. The estimated annual average daily traffic (AADT) in 1992 was 6116 (two way) of which 49.9% was on the GPS direction. The truck traffic in the GPS direction carried approximately 7.9% of the total AADT. The estimate of annual ESALS in the GPS direction using vehicle ESALS is 52.3. These figures are based on 357 days of AVC coverage and 359 days of WIM data in 1992.

Installation of the instrumentation was a cooperative effort between Vermont Agency of Transportation (VAOT) Materials and Research Division, Federal Highway Administration Long Term Pavement Performance Division, and Pavement Management Systems Limited (PMSL) LTPP North Atlantic Region Coordination Office staff. The following personnel participated in the instrumentation installation:

Robert Cauley (LTPP Coordinator)

Sven Coenye Chris Benda Reg Holt Dale Warren Robert McGlynn Alan McBean Roger Lyon-Surrey

Roger Lyon-Surrey Martin Kelley

Dave Blackmore Jim Mobbs

Chuck Booska Bruce King

Bruce Carter

Martin Dukter

John Klemunes

Brandt Henderson Perry Zabaldo Mike Zawisa Doug Marshall VAOT - Materials and Research VAOT - Materials and Research

VAOT - Materials and Research VAOT - Materials and Research VAOT - Materials and Research

VAOT - FWD Testing

VAOT - FWD Testing VAOT - District 5 VAOT - District 5 VAOT - District 5 VAOT - District 5

VAOT - District 5 E.F. Wall Assoc. - Barre, VT

FHWA-LTPP Division

Pavement Management Systems (NARO) Pavement Management Systems (NARO) Pavement Management Systems (NARO) Pavement Management Systems (NARO)

Table 1. Material Properties

Description	Surface	Base	Subgrade
Material	Dense Graded	Crushed Gravel	Silty Sand Gravel
(Code)	HMAC (01)	(304)	(255)
Thickness (mm)	216	655	
Lab Max Dry Density (kg/m³)		2302	2198
Lab Opt Moisture Content (%)		7.00	6.50
In-situ Wet Density (kg/m³) *		2073	2004
In-situ Dry Density (kg/m³) *		2092	1761
In-situ Moisture Content (%) *		2.10	13.98
Bulk Specific Gravity	2.42		
Max Specific Gravity	2.51		
Liquid Limit		0	0
Plastic Limit		0	0
Plasticity Index		NP	NP
% Passing # 200		3.05	6.90

Note: Test pit @ station 5+60

II. Instrumentation Installation

Site Inspection and Meeting with Highway Agency

The site initially nominated for inclusion in the seasonal monitoring program by Vermont was 501683, on state route 7 southbound in North Charlotte. This site was rehabilitated (mill and overlay 10/21/91) but was considered on the basis that the existing available LTPP sites were already instrumented and would be supplemental sites. Prior to finalizing plans for the installation, we were notified by FHWA (June 93) that this site could not be included in the core experiment as it would introduce another factor (rehabilitation) into the experiment. As it was desirable to have a site in Vermont, the decision was to instrument one of the existing supplemental seasonal monitoring sites for inclusion in the core experiment. The New Haven LTPP 501002 site was to be instrumented at the 0+00 end, which would allow for a comparison of the environmental results from the LTPP and state installed instrumentation along with any potential local differences over the length of the site.

As time was limited, a pre survey of this section was not undertaken. Previous surveys did not indicate the section was very uniform, but there were no indication that any major problem may exist. A preliminary planning meeting was held at the Materials and Research Division in Berlin, Vermont on September 8, 1993. The attendees at the meeting were:

Richard Haupt VAOT Research and Technology Engineer

Sven Coenye
 Chris Benda
 VAOT Instrumentation Technician
 VAOT Soils and Foundations Engineer

Larry Willoy
 VAOT Materials Engineer

Reg Holt VAOT Soils Drilling Supervisor

Vincent Janoo CRREL

• Erik Simonsen CRREL (Visiting Ph.D. Student)

Bill Phang
 Brandt Henderson
 Pavement Management Systems, NARO
 Pavement Management Systems, NARO

A presentation on the installation of seasonal monitoring instrumentation and monitoring requirements were provided by Bill Phang and Brandt Henderson of Pavement Management Systems. This was followed by a review and discussion on the seasonal site near New Haven. Plans for the installation on October 6 and October 7, 1993 were discussed; which covered tasks to be done by state resources and material requirements. Correspondence from the site inspection and planning meeting are in Appendix B.

Due to the familiarity of the Vermont VAOT personnel with the requirements for installation of the seasonal instrumentation, a pre installation meeting was not necessary. Arrangements were made to meet on site at 0800 hours with traffic control to be in place by 0830 hours.

Equipment Installed

The equipment installed at the test site included instrumentation for measuring air and subsurface temperature, subsurface moisture content, frost depth, and water table. An equipment cabinet was installed to hold the datalogger, battery pack, and all electrical connections from the instrumentation. The equipment installed are shown in Table 2.

Table 2. Equipment Installed

Equipment	Quantity	Serial Number
Instrumentation Hole		
MRC Thermistor Probe	1	50AT
CRREL Resistivity Probe	1	50AR
TDR Probes	10	50A01-50A10
Equipment Cabinet	-	
Campbell Scientific CR10 Datalogger	1	16556
Campbell Scientific PS12 Power Supply	1	5620
Weather Station		
TE525MM Tipping Bucket Rain Gage	1	12080-693
Campbell Scientific 107-L Air Temperature Probe	1	50AAT
Observation Well/Bench Mark	1	N/A

Equipment Check/Calibration

Prior to installation, each measurement instrument was checked or calibrated. The tipping bucket rain gauge was connected to the CR10 datalogger for calibration. A plastic container with 473 ml of water was placed in the tipping bucket. The container had a small hole in the bottom, which allowed all the water to be drained out in 45 minutes. For the 473 ml of water, the tipping bucket should measure 100 tips \pm 3 tips. The results showed 100 tips, which was within specification.

The air temperature and thermistor probes were connected to the CR10 datalogger simultaneously. They were checked by placing the probes in ice, room temperature, and boiling water. In order for the probes to pass this check, the temperatures for each probe should correspond to the water temperature. The check indicated that the air temperature and thermistor probes were working properly. A second check was done where the air temperature and thermistor probes were connected to the datalogger and run, in air, for 24 hours. The minimum, maximum, and mean temperature for each sensor were checked. All 18 thermistors were similar in their minimum, maximum, and mean readings respectively, therefore the probes were considered functioning correctly. The results of the air temperature and thermistor probes along with the spacing between the thermistors are presented in Appendix B.

The wiring of the resistivity probe was checked using continuity measurements between each electrode and the corresponding pins on the connector. The distance between each

electrode was measured and recorded as shown in Table B-4 in Appendix B. Contact resistance measurements were performed with the probe immersed in a salt water bath. The results of these measurements are also shown in Appendix B. Due to defects in the manufacturing, clear silicon sealant was used to cover exposed wires to the electrodes. The checks on the resistivity probe indicated all electrodes were functioning properly.

The functioning of the TDR probes were checked by performing measurements in air, water, methyl alcohol, and with the prongs shorted at the circuit board and the end of the probe. The traces were taken and the dielectric constant was calculated for the water, air, and methyl alcohol. These values were checked against expected dielectric constants for each medium. The test indicated that all probes were functioning properly. Results of the TDR measurements are presented in Appendix B.

Equipment Installation

Final details for the installation and initial monitoring were discussed with Sven Coenye on the afternoon of October 5, 1993. The installation was confirmed for 0830 hours on October 6, 1993. Traffic control for the installation and monitoring was provided by Vermont Agency of Transportation district 5 maintenance facility in Burlington. The pavement surface drilling and augering of the piezometer and instrumentation hole were done by agency equipment and drilling crew under the supervision of Chris Benda, P.E., Soils and Foundations Engineer. The sawing of the trench and cut for the pavement surface temperature probe were done by Martin Dukter, E.F. Wall and Associates, Barre, VT. The installation of the measurement equipment, the observation piezometer, weather station pole, and cabinet was performed by PMSL staff. Assistance was provided by VAOT Materials and Research Division and the local district personnel.

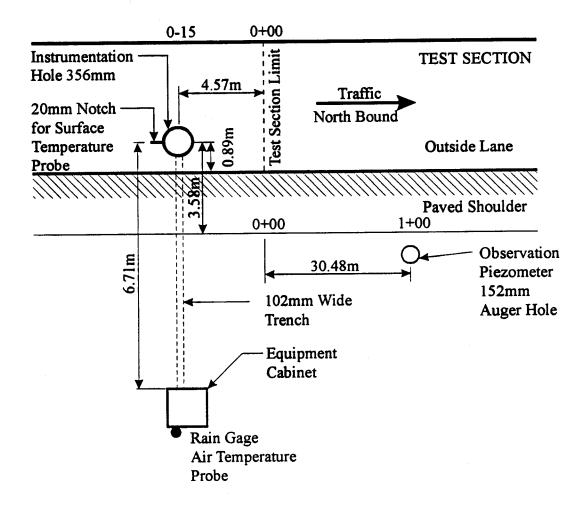
The instrumentation was installed on the south end of GPS 501002, in the northbound lane of route 7, 0.16 km south of junction with route 17 in New Haven, Vermont. The combination benchmark/piezometer was placed in the shoulder at station 1+00. The inpavement instrumentation was installed in the outer wheel path at station 0-15. The cabling from the instrumentation was placed in a 51 mm flexible conduit and buried in a trench running from the instrument hole to an equipment cabinet installed on the slope of the roadway embankment, 6.71 m from the instrumentation hole. To support the cabinet, approximately 0.5 cubic meter of crushed gravel was spread around the cabinet base, which resided on the inside slope of the ditch line. This material would only provide a minor restriction to the water flow in the ditch. This also provides a level surface for access to the cabinet. The weather pole was installed immediately behind the equipment cabinet. Figure 1 provides the location and distances for the various instrumentation and equipment installed.

The installation generally followed the procedures described in the "LTPP Seasonal Monitoring Program: Instrumentation Installation and Data Collection Guidelines". The combination piezometer/benchmark was installed just off the edge of the paved shoulder to a depth of 4.88 m. A 152 mm flight auger was used for drilling the hole. A sample of the material was retained from approximately 1.0 to 2.0 m below the surface. No water

was encountered during the drilling exercise. The piezometer/benchmark was to reside in a soft clay material. To ensure the stability of the pipe assembly, a 300 mm Rocktest borris point was attached to the 25.4 mm galvanized pipe prior to installation. When the pipe was firmly seated in the hole, the borris points were expanded into the clay material to form a firm grip at the base of the pipe. At the completion of the installation of the pipe, 2.75 m of Ottawa sand was compacted at the base of the pipe to provide a filter medium for the water, followed by a 0.35 m of bentonite plug with the remainder of the hole filled with native material. The final elevation for the pipe was 102 mm below the natural ground level at the location of the installation. A CT type cover, held in location by approximately 25 kg of concrete mix, was used to cover and protect the piezometer/benchmark.

A core hole was drilled in the pavement surface, located in the outside wheel path, 0.89 m from the edge of the travel lane at station 0-15, using a 356 mm thin wall diamond core barrel, attached to the truck mounted drilling unit. A 102 mm wide by 220 mm deep saw cut was done between the core hole and the edge of the pavement, using a heavy duty pavement sawing machine. The blade of the pavement saw was used to notch a location for the pavement surface temperature probe at the south edge of the core hole. The remainder of the material from the trench was removed with a pick and shovel.

A combination of methods were used to excavate the instrumentation hole. The crushed dolomite base was scraped out with a hand shovel. The driller used a 280 mm flight auger with a 300 mm cutting tip to loosen the subbase material, which again was removed by The subbase material appeared to be an old road bed that was scarified and blended, as chunks of asphalt concrete were evident throughout the material layer. The underlying clay material was removed in 0.3 to 0.4 m lifts. Care was taken to ensure material removed was consistently representative of material at the location from the bore hole. The findings from the excavation of the instrumentation hole at station 0-15 are presented in Figure 2. All the material excavated from the instrument hole was placed and compacted in order of removal. Samples of the material placed around the TDR probes were retrieved to determine the gravimetric moisture at these locations. A field moisture determination was done at the site with sample material retained for laboratory moisture determination by the VAOT Materials Laboratory. No additional material remained from the instrumentation hole with some material from the trench area required to top up and level the instrumentation hole. The equipment cabinet and pole for the rain gage and air temperature probe were installed as per manual guidelines with the following two exceptions, first the cabinet was installed on the inside ledge of the ditch and a crushed granular fill was used to support the cabinet and provided a platform for access, and second the pole for the rain gage and air temperature probe was installed to a depth of 0.66 m supported in the granular base for the cabinet. The excavation of the trench went fairly smooth as the material was a generally clean sand without cobbles or boulders. The wiring of the instrumentation to the equipment cabinet was completed on the same day as installed.



Height of Air Temperature Probe (center): 2.77m
Height of Tipping Bucket Rain Gage (center): 2.67m
Total Depth of Piezometer: 4.88m
Distance of Piezometer Below Ground Level: 102mm

Figure 1. Location for Seasonal Monitoring Instrumentation Installed at GPS 501002

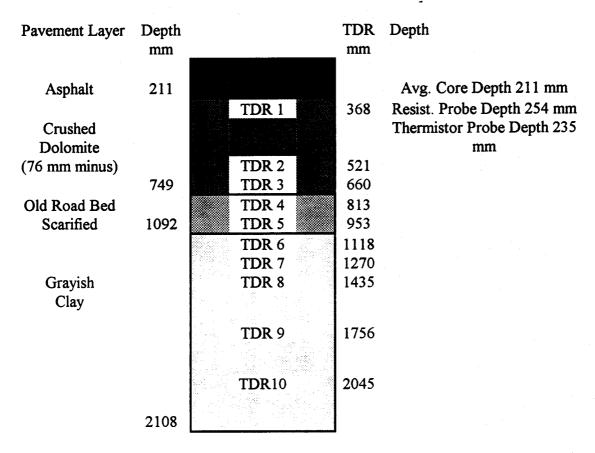


Figure 2. Profile of Pavement Structure and Probe Depths from Surface, Station 0-15

To check for breakage of the TDR probes during installation, each probe was connected to the cable tester and its wave form monitored during compaction of the material around it. The TDR traces are included in Appendix C. By alternating the TDR probes within the instrument hole we were able to keep the cables separate to avoid water from migrating along a bundle of cables attached to the probes placed at various depths. The thermistor and resistivity probes were installed at opposite sides of the instrumentation hole with the thermistor probe 0.235 m and the resistivity probe 0.254 m below the pavement surface. The cables were kept spaced as best as possible until they converged at the opening of the flexible conduit pipe, placed about 50 mm from the edge of the core hole. The cables were then tie wrapped and passed through the conduit to the equipment cabinet. The ends of the conduit were plugged with a mastic pipe sealant.

Tables 3, 4, and 5 present the installed depths of the TDR probes, thermistor sensors, and the resistivity probe respectively. Table 6 gives TDR, field, and laboratory measured moisture content during installation. A comparison of the moisture content from the TDR traces, field, and laboratory determination indicate some discrepancies. The field and laboratory moisture contents generally compare more favorably. The TDR method indicates higher moisture content for the dolomite base material, lower moisture content for the granular subbase, with generally similar values for the clay subgrade with the

exception being sensor 10 at a depth of 2.045 m which is roughly half the moisture content of the field and laboratory results. Given the method under which the material was sampled and the variability in the material, the results can be considered reasonable. It should be noted that the calculation of moisture is dependent on the calibration inputs to the TDR model. Differences of moisture content in the range of 1 to 2% are not uncommon.

Table 3. Installed Depths of TDR Sensors

Sensor #	Depth from Pavement Surface (m)	Layer
50A01	0.368	Base
50A02	0.521	
50A03	0.660	
50A04	0.813	Subbase
50A05	0.953	
50A06	1.118	Subgrade
50A07	1.270	
50A08	1.435	
50A09	1.756	
50A10	2.045	

Table 4. Installed Location of MRC Thermistor Sensor

Unit	Channel Number	Depth from Pavement Surface (m)	Remarks
1	1	0.032	This unit was installed
	2	0.106	in the AC layer.
	3	0.186	
2	4	0.252	This unit was installed
	5	0.328	below the AC layer
:	6	0.404	into the subgrade.
	7	0.481	
	8	0.557	
	9	0.710	
	10	0.859	
	11	1.013	
	12	1.163	
	13	1.315	
	14	1.474	
	15	1.621	
	16	1.780	
	17	1.926	
	18	2.076	

Table 5. Location of Electrodes of the Resistivity Probe

Connector Pin Number	Electrode Number	Depth from Pavement Surface (m)
36	1	0.283
35	2	0.335
34	3	0.385
33	4	0.435
32	5	0.487
31	6	0.536
30	7	0.588
29	8	0.637
28	9	0.687
27	10	0.741
26	11	0.791
25	12	0,841
24	13	0.890
23	14	0.941
22	15	0.992
21	16	1.042
20	17	1.095
19	18	1.145
18	19	1.197
17	20	1.248
16	21	1.298
15	22	1.349
14	23	1.399
13	24	1.448
12	25	1.499
11	26	1.550
10	27	1.602
9	28	1.651
8	29	1.701
7	30	1.751
6	31	1.803
5	32	1.853
4	33	1.904
3	34	1.954
2	35	2.004
1	36	2.053

Table 6. TDR, Field, and Laboratory Moisture Content During Installation

Sensor Number	Sensor Depth (m)	Layer	TDR Moisture Content (by wt)*	Field Moisture Content (by wt)*	Lab Moisture Content (by wt)*
50A01	0.368	Base	3.91%	1.31%	2.20%
50A02	0.521		5.73%	2.03%	1.63%
50A03	0.660		5.73%	2.94%	1.48%
50A04	0.813	Subbase	7.81%	**	**
50A05	0.953	1	9.78%	12.03%	11.53%
50A06	1.118	Subgrade	10.08%	12.99%	13.79%
50A07	1.270		17.95%	19.27%	22.31%
50A08	1.435		16.12%	13.56%	19.70%
50A09	1.756]	20.74%	25.39%	25.45%
50A10	2.045]	12.73%	24.88%	23.67%

* Note: Raw data given in Appendix C

Site Repair and Cleanup

The instrumentation hole was repaired by reinstalling the 356 mm core. Some juggling was required to get the core level with the existing pavement surface. Once the core was leveled it was removed from the hole and the bottom 100 mm was heavily covered with a two part epoxy (PC-7) and reset into the hole forcing the epoxy against the side and up along the wall of the hole. The weight of the state dump truck, which slowly moved back and forth over the core, was used to firmly seat the core into the hole.

The trench for the cabling from the instrumentation hole to the edge of pavement was leveled with crushed gravel to the existing bottom of the paved layer and a cold mix was compacted to the level of the existing surface. The remainder of the trench was filled with a combination of gravel and native material and compacted, followed by a cleanup of loose material from paved area. Traffic control was removed at 1730 hours and the lane reopened to traffic. During the next day the instrument hole and edge of the trench were sealed using Corning self-leveling 888 crack sealing compound. Removal of the asphalt trench material and other disposable items were handled by the VAOT district 5 maintenance crew.

Patch/Repair Area Assessment

When the site was visited on December 20, 1993, two and a half months after installation, the instrumentation hole patch was checked and a photo was taken as shown in Appendix E. The pavement core was slightly below the existing pavement and some settlement had taken place along the trench leading from the instrument hole to the edge of pavement. Additionally, the sealant failed to fully bond to the existing pavement and core.

^{**}Note: No field and lab moisture for probe #4

III. Initial Data Collection

The second day activities included initial data collection on the site and checks on functioning of installed equipment. This consisted of examination of the data collected over the day by the onsite datalogger, data collection and check of the mobile CR10 datalogger, deflection testing, and elevation survey.

Air Temperature, Subsurface Temperature, Rain-fall Data

The air temperature, pavement subsurface temperature profile, and rainfall data, collected on October 7 by the CR10 datalogger, were examined. The equipment and datalogger appeared to be functioning properly. The battery voltages were checked and found to be acceptable. Raw data collected at the site are presented in Appendix D.

The data collected for October 7, 1993 was not sufficient to show change in temperatures. This was due to the fact that the onsite.dld program, that was downloaded to the datalogger, only records hourly averages (only field 5 data collected). The d_onsite.dld, which stores data every minute, should have been used to collect the data.

The tipping bucket rain gauge was checked by determining the number of tips recorded from 473 ml of water discharged into the gauge over a 1 hour time period. The rain gauge was found to be operating properly.

TDR Measurements

TDR data were collected using the mobile system provided by FHWA. The mobile system contains a CR10 datalogger, battery pack, two TDR multiplexers, and a resistance multiplexer circuit board. Version 1.0 of the MOBILE program was used to collect and record the TDR wave form traced for each sensor.

Figures D-1 and D-2 show the initial TDR wave form traces collected with the MOBILE data acquisition system for all 10 sensors. The figures indicate that the multiplexers of the mobile system and TDR sensors were working properly.

Resistance Measurement Data

Resistance data were collected in two modes, automated and manual. The MOBILE data acquisition system automatically performs two point contact resistance measurements and stores the values in terms of millivolts between adjacent electrodes. Figure D-3 shows pavement depth versus measured voltage produced by the MOBILE system.

Manual contact resistance and resistivity measurements were performed using a Simpson Model 420d function generator, two Fluke 87 digital multimeters, to measure voltage and amperage, and a manual electrode switching board. The measured contact resistance data are plotted in Figure D-4 and in Figure D-5 for the 4-point resistivity. Tables D-1 and D-2 in Appendix D show the raw data for the 2-point and the 4-point resistance respectively.

Comparison between Figure D-3 (contact resistance results from automated mode) and Figure D-4 (contact resistance results from manual mode) indicate similar results with the electrodes in the base material indicating differences in resistance from the subbase/subgrade material. Figure D-5 (4-point resistance results from manual mode) indicates that the 4-point test setup has similar results. If the reading equipment is working correctly, all the resistance/resistivity outputs will have similar shapes, although the values will be somewhat different based on the energy applied and distance between sampled electrodes.

Deflection Measurement Data

Deflection measurements followed procedures described in the "LTPP Seasonal Monitoring Program: Instrumentation Installation and Data Collection Guidelines". The analysis results from the FWDCHECK program from the day of installation and the following day are presented in Appendix D. Since then, fifteen more measurements have been collected with the FWD, the first and second on November 8 and December 20, 1993, then on January 12, March 2, March 22, April 13, May 4, May 25, June 22, July 20, August 17, September 21, October 19, November 16, and the fifteenth on December 14, 1994.

Longitudinal Profile Data

According to the guidelines, since this is in a frost area, the survey should be performed on five different occasions; one survey during the middle of each season and one survey during the late winter period (fully frozen condition). Four surveys have already been performed on this site, the first during the fully frozen condition (February 18, 1994), the second in the spring season (April 12, 1994), the third in the summer season (July 25, 1994), and the fourth during the fall season (October 23, 1994).

Elevation Surveys

One set of the surface elevation survey was performed following the guidelines. It was assumed that the elevation at the top of the piezometer pipe was 1.000 meters. The survey was conducted on October 7, 1993 and the results are presented in Appendix D. Since then, five more sets of the surface elevation surveys have been performed, the first on January 12, then on April 13, July 20, August 17, and the fifth on October 19, 1994.

Water Depth

A check of the piezometer indicated that there was no water present. This was odd as water was present in the existing monitoring well at approximately 1.2 m below surface. This would be somewhat consistent with the water evident in the ditch line. Also during our visit of December 20, 1993, water was detected at approximately 1.0 m below surface.

IV. Summary

The installation of the seasonal monitoring instrumentation at the GPS site 501002 near New Haven, VT was completed on October 6, 1993. A check of the equipment and initial data collection was completed on October 7, 1993. The instrumentation, permanently installed at the site, were:

- Time domain reflectometer probes for moisture measurements
- Electric resistivity probes for frost location
- Thermistor probes for soil gradient temperature measurements
- Air temperature thermistor probe and tipping bucket rain gage to record local climatic conditions, and
- Combination piezometer (well) and bench mark to determine changes in water level and pavement elevations.

The pavement gradient temperature and local climatic data are to have continuous data collection stored in an on-site datalogger. The moisture and electrical resistivity are to be collected during each site visit (14 times per year) using a mobile datalogger system. The water level and elevation data are to be collected manually during site visits.

The north end of the site has been instrumented by the VAOT Materials and Research Division. The instrumentation installed were:

- TDR and Watermark moisture sensors
- MRC temperature thermistor probe
- Pavement temperature thermistors
- Ground water level monitoring well with pressure transducer to monitor water table levels
- Air temperature thermistor probe
- Tipping bucket rain gage, and
- Wind speed and direction recorder.

These instruments are connected to a Campbell Scientific datalogger/storage module for continuous data collection, storage, and retrieval. This module is tied into a telephone line for dial-up retrieval of data. Plans are to compare the data from the two sets of instrumentation installed at either end of the site for efficiency of instrumentations and variability/similarity in site data.

The test section is on northbound route 7, 0.16 km south of the intersection of route 17, New Haven. The site is located in a low lying area between two hills. Open fields exist on either side of the pavement structure. The pavement resides in a slightly elevated platform and consists of two 3.7 m wide lanes with 2.44 m paved shoulder. The ditch line is very wet and contains cattails, reeds, and other vegetation. The pavement structure consists of 211 mm of asphalt concrete over 538 mm of 76 mm minus crushed dolomite rock and 343 mm soil aggregate mixture, which appears to comprise part of the old road

bed as there were chunks of asphalt concrete mixed in with the aggregate material. This resides on a grayish clay (A-7 clay). The results are inconsistent with the drilling and sampling logs from 1989, although they are consistent with the findings from the installation of the instrumentation at the north end by VAOT.

All instrumentation was checked prior to installation at the PMSL facility in Amherst, NY. These initial checks indicated that the instrumentation was within specifications, as required for the seasonal monitoring program. Operational checks during installation and the following day indicated that all instrumentation was functioning properly. The air temperature and gradient temperatures measured in the pavement surface compared favorably with the hand held Omega temperature gage. The temperature profile for the pavement soils appeared reasonable with no outlying sensors. A check of the tipping bucket indicated it was functioning correctly with tips corresponding to amount of water supplied.

The resistance/resistivity measurements appeared reasonable with results being typical of traces for similar materials. Moisture content of the soil was determined by TDR method, field moisture determination at time of installation by soil drying, and laboratory results provided by Chris Benda, VAOT Materials and Research Division. There were differences between the moisture content determined by the TDR method and gravimetric moisture content determined from the samples taken, although there was nothing that may indicate the TDR probes were not functioning correctly. Some of the differences may be attributed to the estimated in-situ dry density of the material.

The installation generally went as expected with only a few minor problems. The installation was completed and the section was opened to traffic by 1730 hours. The removal/replacement of the material from the instrumentation hole went very well, with the material being well consolidated around the instrumentation and the core level with the existing pavement surface at completion. It was a surprise that no water was encountered in the piezometer hole although the material was quite saturated.

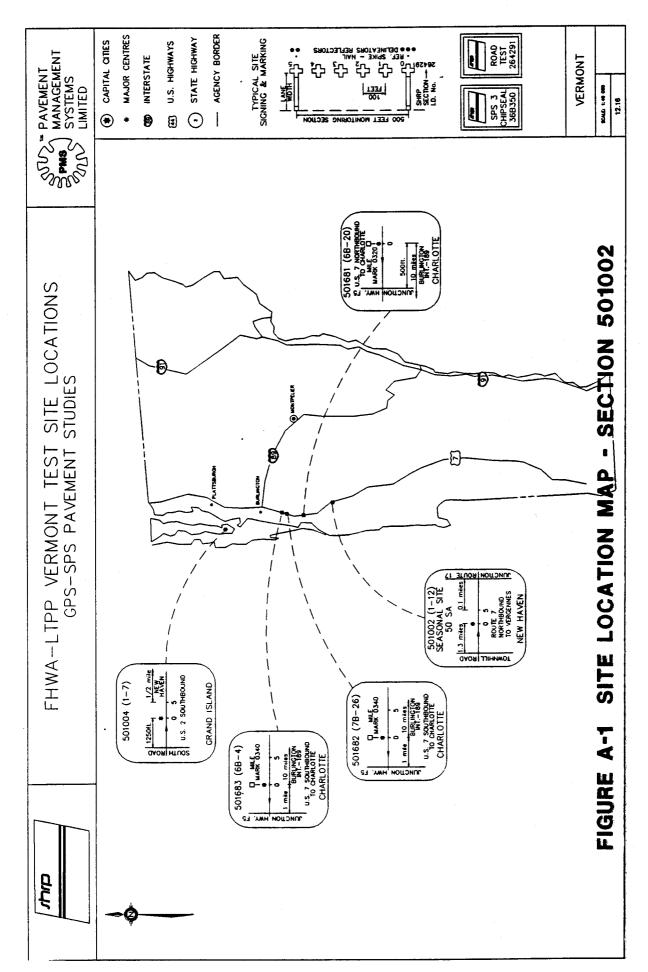
The ongoing monitoring of this section, except for the problems encountered due to weather and technical difficulties with the FWD, has gone fairly well. The state FWD has visited the site on a number of occasions in conjunction with the FHWA-LTPP FWD to collect data for comparison/evaluation purposes.

APPENDIX A

Test Section Background Information

Appendix A contains the following supporting information:

Figure A-1	Site Location Map
Figure A-2	Profile of Pavement Structure
Table A-1	Site Performance Summary
Table A-2	Uniformity Survey Results
Figure A-3	Deflection Profiles from FWDCHECK (Test Date September 7, 1989)
Table A-3	Subgrade Modulus and Structural Number from FWDCHECK (Test Date September 7, 1989)
Figure A-4	Deflection and Temperature Variations with Time, on Site 501002, as Recorded During FWD Testing on April 3, 1991 (State Experiment)
Figure A-5	Deflection and Temperature Variations with Time, on Town Higway 27, as Recorded During FWD Testing on April 4, 1991 (State Experiment)



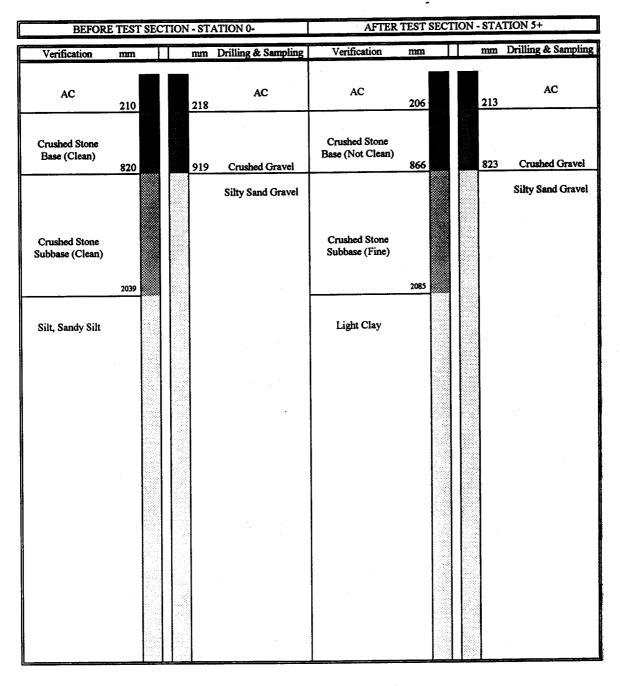


Figure A-2. Profile of Pavement Structure

Table A-1. Site Performance Summary

Distress and Profile Summary						
Distress Summary	Profile S	Summary				
1990	Date (mm-dd-yy)	IRI (in/mi)				
Low Sev. Long. Cracks - 9.31 ft.	10-22-89	77.98				
	06-07-90	77.44				
	05-09-91	68.05				
	07-23-92	70.72				
	08-09-93	83.12				

Falling Weight Deflectometer Data Summary

Date	Mea	an Value for I				
	Sensor 1	Sensor 1 std. dev.	Sensor 7	Sensor 7 std. dev.	Mean Temp D1 (F)	Min/Max TempD1(F)
09-07-89	10.65	2.05	1.81	0.21	82	71/91

	Effective	SN	Subgrade	Modulus	Test Pit	Mod. (psi)
	SN	std dev	Modulus (psi)	std dev (psi)	1	2
09-07-89	8.32	0.20	23444	1825	26468	15282
0, 0, 0,	7.99	0.24	14958	3046		
	7.41	0.10	13738	817		

Note: FWD subsection boundaries at 210 ft and 335 ft as entered into RIMS.

Table A-2. Uniformity Survey Results

Seasonal Uniformity Survey Site Number: 501002 Date Surveyed: September 7, 1989					Falling Weight Deflectometer Data Collection and Processing Summary			
Section Mean Deflection Values for HT 2 (mils) - Corrected (ft)								
	Sensor 1	Sensor 1 std dev	Sensor 7	Sensor 7 std dev	Subg modulus (psi)	Subg modulus std dev	Effective SN	SN std dev
-50					26239		9.45	
0 -310	8.77	0.91	1.71	0.21	22057	3009	8.41	0.26
310 -500	12.24	0.34	1.95	0.08	16231	1215	7.48	0.10
562					18514		8.00	

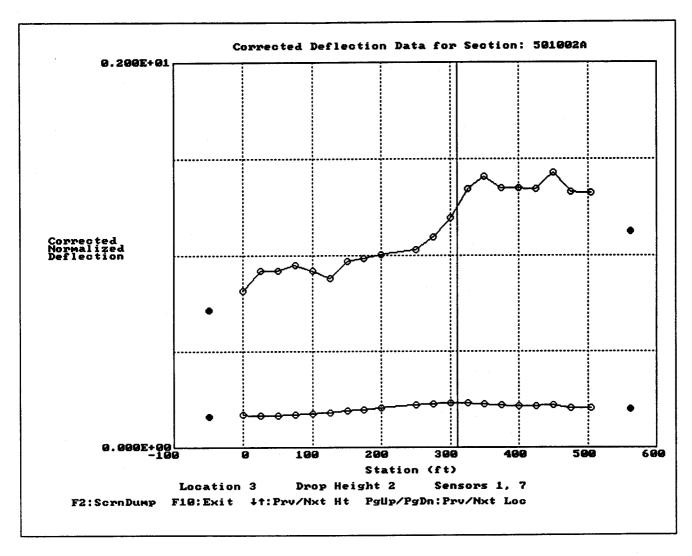


Figure A-3. Deflection Profile from FWDCHECK (Test Date September 7, 1989)

Table A-3. Subgrade Modulus and Structural Number from FWDCHECK (Test Date September 7, 1989)

Flexible 1	Pavement Thickness Stat	istics - 501002A - Drop	Height 2
Subsection	Station	Subgrade Modulus	Effective SN
Test Pit	-50	26239	9.45
1	0	24756	8.95
	25	25205	8.45
	50	25051	8.45
	75	24672	8.35
	100	24034	8.50
	125	23341	8.75
	150	22095	8.40
	175	21534	8.35
	200	20240	8.35
	250	19289	8.30
	275	18510	8.10
-	300	15955	7.95
2	325	14386	7.65
	350	14593	7.45
	375	15791	7.55
	400	17290	7.45
	425	16287	7.50
	450	16774	7.30
	475	17540	7.45
	500	17189	7.50
Test Pit	562	18514	8.00
Subsection 1	Overall Mean	22057	8.41
	Standard Deviation	3009	0.26
	Coeff of Variation	13.64%	3.10%
Subsection 2	Overall Mean	16231	7.48
	Standard Deviation	1215	0.10
	Coeff of Variation	7.48%	1.33%

Note: Station 225 not tested

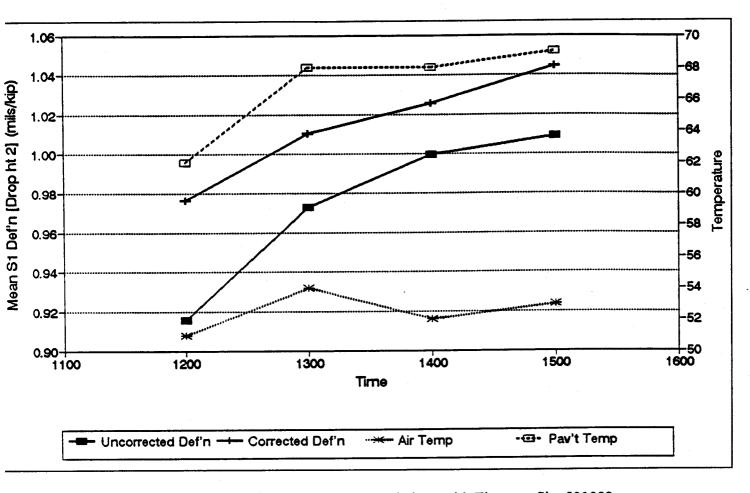


Figure A-4. Deflection and Temperature Variations with Time, on Site 501002, as Recorded During FWD Testing on April 3, 1991 (State Experiment)

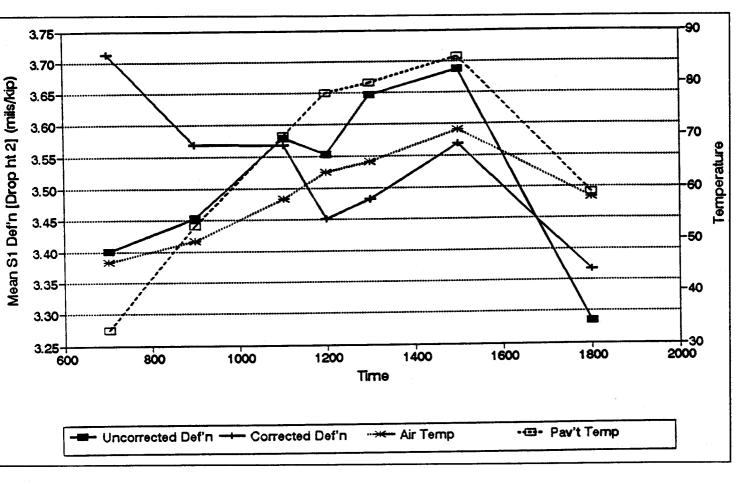


Figure A-5. Deflection and Temperature Variations with Time, on Town Highway 27, as Recorded During FWD Testing on April 4, 1991 (State Experiment)

APPENDIX B

Supporting Site Visit and Installed Instrument Information

Appendix B contains the following supporting information:

Correspondence from the Site Inspection and the Planning Meeting

- Table B-1. Air Temperature Thermistor Calibration
- Table B-2. MRC Probe Calibration
- Table B-3. Description of MRC Thermistor Probe and Sensor Spacing
- Table B-4. Resistivity Probe and Sensor Spacing
- Table B-5. Contact Resistance Calibration
- Table B-6. TDR Probes Calibration
- Figure B-1. TDR Traces Obtained During Calibration



MEMORANDUM

TO:

Dick Haupt

DATE:

February 14, 1991

FROM:

Bill Phang Bill Hang

FILE REF:

12.16.1

SUBJECT: FWD Seasonal Testing, NARCO Year 1

COPIES TO:

I. Pecnik

B. Henderson

NARCO is developing plans for their Year 1 core experiment. For easy reference enclosed are copies of a NARCO newsletter describing the FWD seasonal testing concept, and a SHRP State participation Synopsis inviting nominations.

As you are aware through discussions with Brandt Henderson, NARCO expects to include three (3) Vermont GPS-1 sites which were instrumented by CRREL in the Year 1 plan. These are GPS 501002, 501004, and 501994. A list of GPS sites in Vermont is attached.

The SHRP seasonal testing plan is not yet finalized, but at present the agency would be responsible for traffic control during FWD testing. The test frequency during the spring thaw period is about once weekly, but reduces during fall and winter to twice per season. A total of from 12-16 test per year per GPS site is expected.

NARCO proposes to initiate a pilot Year 1 program this spring, so would appreciate confirmation of your participation.



MEMORANDUM

TO:

C. Dougan, CT

DATE:

May 07, 1992

T. Karasopolous, ME

L. Kenison, NH R. Cauley, VT

PROJECT:

50450732

G. Jones, ON

G. Dore, QE

P. Hughes, MA

FILE:

6.01

FROM:

BIII Phang Bill thang

SUBJECT:

SHRP Seasonal Monitoring Reconfirming Participation

COPIES TO: See Below

Planning for the SHRP Seasonal Monitoring program has now progressed to the stage where preliminary schedules for installation of temperature, moisture, and frost depth penetration need to be determined.

The results of the measurements made at the pilot seasonal testing site at Syracuse, N.Y., and at Boise, ID, are to be examined and recommendations made regarding the instrumentation which will be used at other seasonal testing sites by the end of May 1992. These recommendations will be discussed and the instrumentation finalized by the Instrumentation ETG in June 1992. Acquisition of equipment and plans for installation over the next few months imply that field installation will begin in September 1992.

in the meantime, in order to develop and test the schedules for FWD testing, a trial run of the testing circuit will be made in July. At this juncture the eight (8) first round GPS seasonal testing sites include:

Cell No.	Agency	SHRP ID	SHRP Expt.	Subgrade	AC Thickness	Traffic	
4 *	ON **	871620	1	Fine	4.5	High	
12	NY **	361011	1	Fine	10.7	Low	
12	VT	510002	1	Fine	8.1 %	Low	
16	CT **	091803	1	Coarse	7.0	Low	251112 25.00 30
16	MA	251003	1	Coarse	8.5	High	251112
16	NH	331001	1	Coarse	8.3	High	And Win
20	QE	893015	3	Fine	8.5	High	in the second
24	ME	233014	3	Coarse	10.0	Low	1311 Million Mills
To be re	ehabilitated is	n May 1992	2	**	1992 Confirm	nation	2.7

415 LAWRENCE BELL DRIVE **UNIT #3** AMHERST, N.Y. 14221

TEL. (716) 632-0804 FAX (716) 632-4808 B-2



MEMORANDUM

TO:

Guy Dore, QE

Dick Haupt, VT

Warren Foster, ME Alan Rawson.. NH

Charles Dougan, CT

FROM:

SUBJECT:

Bill Phang Bill than

Seasonal Testing

Preliminary FWD Investigation

DATE:

FILE:

June 29, 1992

PROJECT:

50450732

6.01

COPIES TO: See Below

One of the findings of the seasonal testing pilot at Syracuse, NY was that the FWD results near the points where the sensors were installed outside of the test section were somewhat different to results within the GPS test section.

It was subsequently decided that for future seasonal testing installations, a preliminary FWD test series would be conducted in the areas adjacent to the test sections. This would help to fix on a location for sensor installation that more closely represents the part of the test section being monitored for seasonal effects.

The FWD has been scheduled to carry out these preliminary tests beginning July 21 at GPS 893015 in Trois Riviere, QE. Other sites are 501683 in Charlotte, VT, on July 24, 237028 in Bethel, ME on July 28, 331001 in Concord, NH on July 29 and 091803 in Groton, CT July 30.

A copy of the FWD schedule is attached. Would you please make necessary arrangements for traffic control. Please call Brandt Henderson of you have any questions.

Distribution to:

I.J. Pecnik

B. Henderson

OFFICE MEMORANDUM

AGENCY OF TRANSPORTATION

TO: Christopher C. Benda, P.E., Soils & Foundations Eng.

FROM: Richard S. Haupt, P.E., Research & Testing Engineer

DATE: July 15, 1992

SUBJECT: Installation of Seasonal Instrumentation @ SHRP Sites

Please schedule the following dates for performing the necessary borings for installation of below ground instrumentation sensors, monitoring wells and posts for instrumentation:

July 30 Thursday
July 31 Friday
August 3 Monday
August 4 Tuesday
August 5 Wednesday
August 6 Thursday
August 7 Friday

All work will be weather dependent. The scheduled work will proceed as follows:

- (1) Three sites on VT 62 (Dist. 6)
- (2) New Haven SHRP site 501002 (Dist. 5)
- (3) Charlotte SHRP site 501681 (Dist. 5)
- (4) South Hero SHRP site 501004 (Dist. 8)

Each site will need traffic control. The work at each location will involve the following:

- (1) Core drilling thru the shoulder pavement at the edge of the traffic pavement with a 15" core bit. Remove pavement core.
- (2) Saw cut a 3 to 4 inch width strip from the hole to the edge of the shoulder. Remove pavement and enough gravel to insert a 2" ID PVC conduit sleeve below the surface of the shoulder pavement.
- (3) With a 14" dia. Auger, carefully remove each layer of subbase materials to the Subgrade. Representative samples of each material to be taken. Moisture samples of representative material to be taken.

- (4) Install a 10 foot long (one on VT 62 to be 5 feet) temperature probe in each core hole plus five moisture sensors at designated depths (75", 51", 39", 27" & 15" depths below surface as the hole is backfilled). Two temperature sensors will also be installed in the pavement in each core hole.
- (5) The 15" diameter core will be replaced and sealed. Hot mix will be needed to fill the slotted hole.
- (6) New monitoring wells will need to be installed at New Haven & Charlotte. I expect that a depth between 10 and 15 feet will be adequate.
- (7) Charge Numbers:
 - (a) Vt. 62 Berlin-Barre F026-1(33) SF 03 Project 3457 FF 50 Task Code 2662
 - (b) New Haven & South Hero Statewide F SHRP (1) SF 03 Project 3414 FF 50 Task Code 2605
 - (c) Charlotte Charlotte F019-4(21)
 SF 03
 Project 3626
 FF 50
 Task Code 2662

I will keep the District personnel informed of the progress as the work proceeds.

RSH:sls

CC: David Blackmore - Dist. 5
Ray Cyr - Dist. 6
Jim Smith - Dist. 8
Vincent Janoo - CRREL
Dale Bull, CRREL
Bill Phang - PMS
Brant Henderson - PMS
Aramis Lopez, FHWA LTPP Div.
R. Cauley
Sven Coenye
RFC/Lab File - Read. File - RSH File - Central Files



STATE OF VERMONT AGENCY OF TRANSPORTATION 133 State Street, Administration Building Montpelier, Vermont 05633 FAX # (802) 828-2792



July 23, 1992

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FILE"			
			

Mr. Bill Phang Pavement Management Systems 415 Lawrence Bell Drive - Unit #3 Amherst, New York 14221

Dear Mr. Phang:

Subject: GPS Seasonal Monitoring Sites

Your questionnaire on seasonal monitoring installations, SHRP GPS seasonal study site number 501683, has been completed and is being returned.

Along with this questionnaire, you will find a copy of a report on soil sample, laboratory number E88721.

Sincerely,

Robert F. Cauley, P.E.

Materials and Research Engineer

best & Cauley

RFC:RSH:etn

Enclosures





STATE OF VERMONT AGENCY OF TRANSPORTATION 133 State Street, Administration Building Montpelier, Vermont 05633

FAX # (802) 828-2792



October 5, 1992

Mr. Aramis Lopez, Jr. Highway Research Engineer U.S. Department of Transp. - FHWA 6300 Georgetown Pike, HNR-40 McLean, Virginia 22101-2296

Dear Mr. Lopez:

I would like to thank you and Brant for your assistance in our installation of the South Hero sensors that will become part of the data collection program for seasonal testing. We appreciated the opportunity to discuss this project with both of you and learn your perspectives for the overall program.

As discussed during your visit, we are in the process of developing a project outline that will describe its overall scope, objectives, and evaluations and what we expect to derive from performing a seasonal testing program. Attached is a draft copy of this project. We would appreciate any input and comments that you or any of your staff would like to contribute.

When you have developed a protocol and process for evaluation of seasonal test data, we would be interested in a copy. meantime, if we can be of any assistance in its development, please let me know. Until a unified protocol has been established, we will do our best to derive what is occurring from the collected data.

As we are able to derive results from the data collected, I will provide you with copies of the information. If there is anything that I can provide for the regional meeting at Hartford on November 5th, please let me know. If there are pictures that need to be made into slides, I would need to know by October 19, 1992:

I look forward to seeing you again on November 5, 1992, and the opportunity for an update on the status of SHRP related activities.

Sincerely,

Richard S. Haupt, P.E.

Vermont SHRP Contact Engineer

RSH:etn

ccB. Henderson, Vermont is an Equal Opportunity Employer, V. Janoo - Dir. Merchant

DATE REC.OCT	-91992
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VERMONT AGENCY OF TRANSPORTATION SHRP-LTPP SITE TESTING

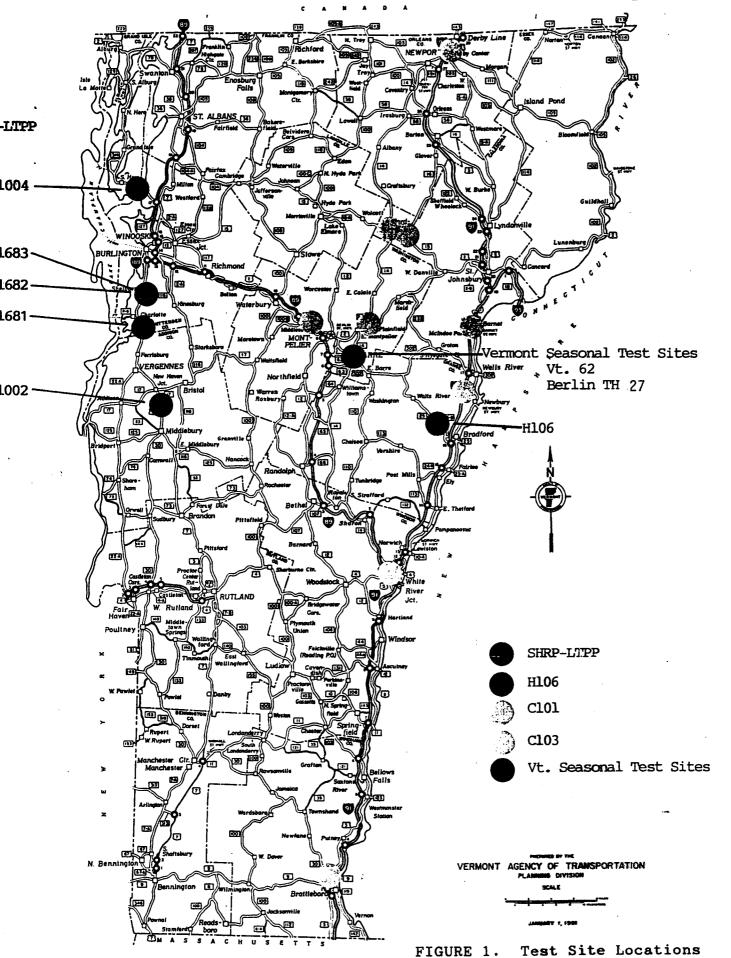
Background

One of the primary goals of the Strategic Highway Research Program (SHRP), now under the jurisdiction of the Federal Highway Administration (FHWA), is to collect and evaluate data that will influence revisions to the AASHTO Design Guide. To help achieve this goal, the Long Term Pavement Performance (LTPP) Program was established. This 20-year program involves testing almost 1,000 in-service sections of pavement representing common types of pavement design currently in use, and sections which highlight certain factors or conditions calling for special design consideration. Among these conditions are severe soil, drainage, or environmental conditions, any of which often result in less than predicted performance.

LTPP sites that highlight these conditions have been identified in Vermont: one on U. S. Route 7 in New Haven, four on U.S. Route 7 in Charlotte, and one on U. S. Route 2 in South Hero. The Materials and Research Division of the Vermont Agency of Transportation (VAOT) is assisting the SHRP study in the evaluation of these sites by monitoring seasonal soil moisture and temperature variations, pavement temperature, loading, and deflections, atmospheric and ground water conditions, and pavement performance.

This monitoring program is an extension and modification of a program initiated in 1989 in conjunction with the U.S. Army Corps of Engineers Cold Regions Research Engineering Laboratory (CRREL) in Hanover, New Hampshire, to evaluate the effects of moisture in and near sections of a highway facility constructed over ledge. A location on Vt. Route 62 in Berlin was initially selected for this study, and this site has been included in the testing program being performed for the SHRP locations. Another test site adjacent to the Materials and Research Division on Town Highway 27 has also been included to evaluate the performance of a thin (weak) pavement section.

The five test locations all differ in traffic loading, pavement structure, and subgrade conditions (both soil type and moisture). The site locations are shown in Figure 1, and the site conditions are shown in Table 1.



B-9

Table 1. Test Site Profiles

Site	Pavement Structure	Subgrade	Traffic
VT 62 (Berlin) 9.5" pavement 24" subbase 18" - 24" sand	silt	4,800 ADT
TH 27 (Berlin) 2.5" pavement 18" gravel	silt	Low
SHRP SITE 501			
New Haven	8" pavement 24" crushed rock	A-7 clay	6,500 ADT
SHRP SITE 501	681		
Charlotte	10" pavement 20" gravel 12" granular borro	3'-9" A-1-a Granular ow silt	11,000 ADT
SHRP SITE 501	004		
South Hero	8" pavement 24" crushed gravel 24" sand	hard pan silt	7,800 ADT
*SHRP SITE 16	83	A-6 clay	11,000 ADT
Charlotte	10" pavement 20" gravel 12" granular borro	ow .	-

^{*}Seasonal Test Site to be Installed in 1993 by SHRP

Objective

The specific objectives of the seasonal testing program are to monitor soil temperatures to a depth below the frost level, monitor moisture of each subbase and subgrade layer, and measure rainfall, air temperature, wind speed and direction, and water table fluctuations at the site year-round. This data will then be combined with traffic data from Weigh-in-Motion (WIM) installations located at each site, pavement deflections measured by FWD testing, and distress surveys to evaluate the effects of the conditions on different pavement structures.

From this testing, the following will be evaluated:

- Effect of freeze/thaw on pavement performance
- Effectiveness of sand layers for drainage
- Length and severity of freeze/thaw seasons
- Sources of moisture in pavement structures, time of retention, and soil permeability
- Relative performance of structural layers (layer coefficients)
- Seasonal effects on the structural capacity of structural layers (layer coefficients)
- Effects of traffic loading on pavement performance under severe moisture and temperature conditions (spring thaw).

All data and information will be used to evaluate and improve pavement design, pavement specifications, and maintenance management criteria. The whole concept of this program is integrated with the SHRP-LTPP program and will become part of their worldwide databases.

Site Installation And Testing

Each location has received essentially the same format for installation of instrumentation, with variations tailored to the differences in pavement structure at that location. This treatment involves the placement of moisture and temperature sensors in the pavement structure and subgrade, soil property characterization, installation of a ground water level monitoring well, and installation of a weather monitoring station. Each site has a Weigh-in-Motion (WIM) installation, except TH 27 where a counter classifier was used due to its low traffic volume.

INSTALLATION: A 15" diameter core was cut through the pavement and removed. An auger was used to remove the material below the pavement, keeping each removed layer separate for replacement. Samples of each material were taken for analysis and testing. Removal of material continued to a depth of 75". At that depth, a four inch diameter hole was drilled through the subgrade to a depth of 11'. A 10' long MRC temperature probe was installed in the 11' hole and backfilled. This probe measures temperature at 3 inch

intervals to a 15 inch depth, and then at one foot intervals to ten feet. TDR and Watermark moisture sensors were placed at depths of 75", 51", 39", 27" and 15" (the New Haven site, due to its reduced pavement section, received sensors at 39", 27", and 15" depths only). Backfilling and compaction was performed using the same material removed from the pavement structure. Thermistor temperature sensors were placed in the pavement at 2" and 6" depths below the surface. All wires were run through a 2" I.D. PVC pipe placed in a trench under the shoulder pavement. The roadway pavement plug and shoulder cut sections were replaced and sealed. Figure 2. depicts a typical site installation.

FIGURE 2. Typical Sensor Installation

Top of Pavement	Sensor	Depths		Shoulder 2 1/2" AC-			
Pavement	+	2"		1			
8 - 10" AC	+	6"					
	_	12"		_1			
•		15"	*				
	_	18"					
		21"					
Subbase	• 🕳	24"					
	-	27"	*				
	_	39"	*				
Sand	-	51"	*				
		63"					
Subgrade	-	75"	*				
_	-	87"					
	-	99"					
·		111"					
		123"					
	-	132"					

- * TDR and Watermark moisture sensors at each depth indicated.
- MRC Temperature thermistor probe is from 12" to 132" below top of pavement with a thermistor at each depth indicated.
- + Pavement temperature thermistors at each indicated depth.

A ground water level monitoring well was drilled approximately 15' from the data collection recorder and equipped with a pressure transducer to monitor water table levels. A weather monitoring station, including a rain gauge, wind speed and direction gauge, and temperature sensor, was also installed adjacent to the recorder. The temperature sensors, moisture sensors, groundwater elevation, wind speed, wind direction, rainfall, and pavement temperature sensors are connected to a Campbell Scientific control module for continuous data collection, storage and retrieval. This module is tied in to a telephone line for dial-up retrieval of data. The TH 27 site has a continuous recorder for the TDR soil moisture probes. Due to equipment costs, all other sites are scheduled to be manually recorded on at least a monthly basis.

Deflection data from the FWD will be obtained at each site on a scheduled basis throughout the year. Deflections will be measured monthly during the Winter, twice a month during Summer, and at least weekly during the critical Fall (freeze) and Spring (thaw) seasons. WIM information is part of the required data for indicating traffic classification, loadings, Equivalent Single Axle Loads (ESAL's), and Average Daily Traffic (ADT) counts at each site.

Throughout the year, data will be retrieved and analyzed for each site. The sensors will indicate temperature and moisture conditions at various depths of the pavement structure. This can be correlated to rainfall and/or water table levels and air temperatures. Additionally, pavement performance under various conditions of moisture and temperature (freeze/thaw) can be evaluated through FWD deflections, distress surveys and traffic loading data.

Applications of the Testing

VAOT and SHRP will use the test results to address the following issues:

- Evaluation of the effectiveness of sand layers for drainage
- Identification of length and severity of spring thaw periods (including frost depths, role of thaw and moisture levels)
- Pavement performance during spring thaw periods under different loading conditions
- Identification of sources of moisture in pavement structures and retention times
- Permeability of soil layers and drainage characteristics for use in pavement design equations
- Effectiveness of pavement layers and correlation with structural layer coefficients used in pavement design

- Verification of effective roadbed resilient modulus used in pavement design calculations
- Evaluation of severe climate and moisture conditions on pavement design
- Some of these issues are currently being proposed for inclusion on the FY '93 Highway Planning Research (HPR) work plan. The SHRP testing will support the new initiatives.

Table B-1. Air Temperature Thermistor Calibration

LTP	P Season	al Monite	oring Stud	у	State Code	·]	50]
Air Ter	nperature	Thermis	tor Calibra	ation	Test Section	on Numb	er	[10	02]
Before Operation Checks Calibration Da Probe S/N Operator					te mm-dd-y	/у			4-93 AAT MZ
11	Mobile Datalogger (24 hour)		Water Room Temperature		Ice Bath 0 C (+/- 1 C)		Hot Water 50 C (+/-)		ok
Mean	Min.	Max.	Reading	Time	Reading	Time	Reading	Time	y/n
8.71	3.90	14.85	22.20	1201	0.156	1112	52.80	1243	у

Probe Accepted P.Z. & M.Z. (Initials)

Table B-2. MRC Probe Calibration

LTPP Seasonal Monito	ring Study	State Code	[50]
MRC Probe Calib	ration	Test Section Number	[1002]
Before Operation Checks		Date mm-dd-yy	10-04-93
Before operation entering	Probe S/N		50AT
	Operator		PZ & MZ

		ile Datalo 24 hour		Water Room Temp Time 1201	Ice Bath 0 C(+/- 1 C) Time 1112	Hot Water 50 C (+/-) Time 1243	ok
No.	Mean	Min.	Max.	Reading	Reading	Reading	y/n
1	8.70	3.91	14.71	22.4	0.81	53.6	у
2	8.71	3.92	14.68	22.3	0.63	53.7	у
3	8.80	3.99	14.73	22.4	0.99	53.4	у
4	8.76	4.11	15.01	22.7	0.88	50.8	у
5	8.78	4.08	14.96	23.0	0.45	53.5	у
6	8.66	3.94	14.87	23.0	0.26	53.4	у
7	8.61	3.87	14.94	23.0	0.33	52.9	у
8	8.53	3.84	14.87	22.9	0.40	53.5	у
9	8.54	3.87	14.90	23.0	0.88	54.2	у
10	8.56	3.85	14.82	22.8	0.99	54.2	у
11	8.56	3.84	14.84	22.8	0.74	54.4	у
12	8.47	3.77	14.73	22.6	0.70	54.2	у
13	8.52	3.77	14.80	22.8	0.59	54.6	у
14	8.52		14.73	22.7	0.73	53.9	у
15	8.52		14.68	22.7	0.70	53.7	у
16	8.54		14.79	22.8	0.56	53.5	у
17	8.14		14.87	22.9	0.56	52.6	у
18	8.02		14.90	22,8	0.25	53.1	у

Probe Accepted:	M.Z.	(Initials)
Probe Length:	1.854	(meters)

Ther	mistor dis	tance fr	om top of	orobe:	• (meters)			
4	0.017	7	0.246	10	0.624	13	1.080	16	1.545
5	0.093	8	0.322	11	0.778	14	1.239	17	1.691
6	0.169	9	0.475	12	0.928	15	1.386	18	1.841

Table B-3. Description of MRC Thermistor Probe and Sensor Spacing

Unit	Channel No.	Distance from Top of Unit(m)	Remarks
1	1	0.013	0.3302 m long by 63.5 mm
	2	0.165	stainless steel probe installed
	3	0.318	in the AC layer.
2	4	0.017	1.854 m long by 25.4 mm
	5	0.093	PVC tube installed
	6	0.169	in the base and subgrade.
	7	0.246	
	8	0.322	
	9	0.475	·
	10	0.624	
	11	0.778	
	12	0.928	
	13	1.080	
	14	1.239	
	15	1.386	
	16	1.545	
	17	1.691	
	18	1.841	

Table B-4. Resistivity Probe and Sensor Spacing

Connector	Electrode	Continuity	Measure-	Spacing (mm)			Dist. from
Pin No.	Number	x	ment	Line 1	Line 2	Avg.	Top (m)
36	1	x	Top-1	29	30	29.5	0.029
35	2	х	1-2	52	51	51.5	0.081
34	3	x	2-3	50	51	50.5	0.131
33	4	х	3-4	50	49	49.5	0.181
32	5	х	4-5	52	53	52.5	0.233
31	6	х	5-6	49	50	49.5	0.282
30	7	х	6-7	52	51	51.5	0.334
29	8	x	7-8	49	48	48.5	0.383
28	9	х	8-9	50	51	50.5	0.433
27	10	х	9-10	54	55	54.5	0.487
26	11	х	10-11	50	50	50.0	0.537
25	12	x	11-12	50	51	50.5	0.587
24	13	x	12-13	49	48	48.5	0.636
23	14	x	13-14	51	52	51.5	0.687
22	15	х	14-15	51	50	50.5	0.738
21	16	x	15-16	50	51	50.5	0.788
20	17	x	16-17	53	52	52.5	0.841
19	18	x	17-18	50	49	49.5	0.991
18	19	х	18-19	52	51	51.5	0.943
17	20	х	19-20	51	52	51.5	0.994
16	21	х	20-21	50	50	50.0	1.044
15	22	х	21-22	51	50	50.5	1.095
14	23	х	22-23	50	50	50.0	1.145
13	24	х	23-24	49	49	49.0	1.194
12	25	х	24-25	51	51	51.0	1.245
11	26	х	25-26	51	52	51.5	1.296
10	27	x	26-27	52	50	51.0	1.348
9	28	х	27-28	49	50	49.5	1.397
8	29	х	28-29	50	50	50.0	1.447
7	30	х	29-30	50	50	50.0	1.497
6	31	х	30-31	52	52	52.0	1.549
5	32	х	31-32	50	51	50.5	1.599
4	33	х	32-33	51	49	50.0	1.650
3	34	х	33-34	50	51	50.5	1.700
2	35	х	34-35	50	51	50.5	1.750
1	36	х	35-36	49	50	49.5	1.799
			36-End	25	25	25.0	1.824

Table B-5. Contact Resistance Calibration

L	TPP Seaso	onal Monite	oring Stud	v	State Cod	le	[50]		
		ata Sheet R		<u> </u>					
		istance Me		· · · · · · · · · · · · · · · · · · ·	Test Section Number [1002				
1. Date (M		***	ALSOI CITION		[10-04-93]				
		ts Began (M	(ilitary)				[1330]		
3. Comme		20 Bus (s	<i>y</i>		In Salt Water Prior to Installatio				
Test	Conne	ctions	Voltage	(ACV)	Current	(ACA)	Notes		
Position	I	I	Range	Reading	Range	Reading			
	V	V	Setting		Setting				
1	1	2	mV	159.4	uA	116.3			
2	3	2	mV	159.1	uA	115.9			
3	3	4	mV	158.2	uA	114.8			
4	5	4	mV	159.2	uA	112.6			
5	5	6	mV	154.5	uA	119.0			
6	7	6	mV	152.2	uA	122.1			
7	7	8	mV	148.1	uA	126.5			
8	9	8	mV	149.9	uA	123.7			
9	9	10	mV	153.0	uA	119.0			
10	11	10	mV	148.7	uA	124.4			
11	11	12	mV	153.2	uA	117.8			
12	13	12	mV	153.6	uA	116.9			
13	13	14	mV	153.9	uA	116.4			
14	15	14	mV	151.9	uA	118.8			
15	15	16	mV	149.8	uA	121.4			
16	17	16	mV	149.3	uA	121.5	***		
17	17	18	mV	146.6	uA	125.1			
18	19	18	mV	148.5	uA	121.9			
19	19	20	mV	146.5	uA	123.8			
20	21	20	mV	144.5	uA	125.8			
21	21	22	mV	144.5	uA	125.4			
22	23	22	mV	145.9	uA	123.9			
23	23	24	mV	144.6	uA	124.8			
24	25	24	mV	144.7	uA	124.2			
25	25	26	mV	145.2	uA	122.5			
26	27	26	mV	146.5	uA	120.9			
27	27	28	mV	145.7	uA	121.4			
28	29	28	mV	144.7	uA	122.4			
29	29	30	mV	142.9	uA	124.6			
30	31	30	mV	143.1	uA	123.9			
31	31	32	mV	142.5	uA	124.1			
32	33	32	mV	141.4	uA	124.9			
33	33	34	mV	140.7	uA	125.6			
34	35	34	mV	140.0	uA	126.4			
35	35	36	mV	138.1	uA	128.8			
36	37	38	mV	<u> </u>	uA				
37	38	39	mV		uA	1			
38	39	40	mV ·	<u> </u>	uA				
Preparer	:	Michael 2	Lawisa		Employe	r :	PMSL		

Table B-6. TDR Probes Calibration

State Code	[50]
Test Section Number	[1002]
	State Code

Before Operation Checks	P.Z.	Initial	Calibration Date (mm-dd-yy)	10-02-93
			Seasonal Site	50SA

				Probe Shorted		Air	Alcohol	Water
	Probe	Resistance	(ohms)	Begin	End	Begin	Begin	Begin
No.	(S/N)	Core	Shield	Length	Length	Length	Length	Length
1	50A01	0.700	1.100	16.331	16.451	16.351	16.361	16.361
2	50A02	0.800	0.700	16.331	16.501	16.341	16.361	16.361
3	50A03	0.700	0.600	16.311	16.451	16.331	16.351	16.351
4	50A04	0.800	0.700	16.341	16.501	16.361	16.361	16.371
5	50A05	0.900	0.800	16.381	16.501	16.351	16.411	16.431
6	50A06	0.600	1.200	16.321	16.351	16.341	16.361	16.361
7	50A07	0.800	0.700	16.391	16.501	16.391	16.431	16.461
8	50A08	0.800	0.600	16.331	16.501	16.351	16.361	16.361
9	50A09	0.600	0.500	16.351	16.481	16.351	16.361	16.481
10	50A10	1.000	0.800	16.421	16.491	16.401	16.411	16.461

NOTE: Record lengths from TDR

Calculation of Dielectric Constant

Probe Length 0.203 m $\epsilon = \boxed{TDRL \over (PL)(V_p)}^2$ V_p Setting 0.99 V_p

		Air			Alcohol	*****		Water	
	TDR	Dielectric	In Spec.	TDR	Dielectric	In Spec.	TDR	Dielectric	In Spec.
No.	Length	Constant	(?)	Length	Constant	(?)	Length	Constant	(?)
1	0.19	0.88	у	1.20	35.0	у	1.86	84.0	y
2	0.20	0.97	у	1.21	35.5	у	1.86	84.0	у
3	0.20	0.97	у	1.21	35.5	у	1.86	84.0	у
4	0.20	0.97	у	1.19	34.4	у	1.87	84.9	у
5	0.21	1.07	у	1.22	36.1	у	1.86	84.0	у
6	0.20	0.97	у	1.20	35.0	у	1.85	83.1	у
7	0.18	0.78	у	1.21	35.5	у	1.86	84.0	у
8	0.19	0.88	у	1.24	37.3	у	1.87	84.9	у
9	0.21	1.07	у	1.20	35.0	у	1.86	84.0	у
10	0.18	0.78	у	1.20	35.0	y	1.85	83.1	y

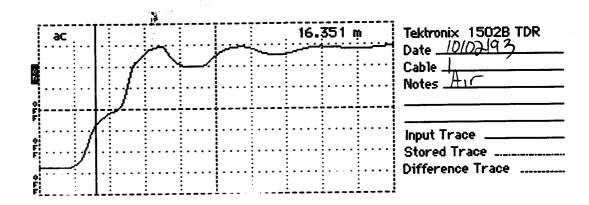
LTPP Seasonal Mon	itoring Study	State Code	
TDR Probe Cal	ibration	Test Section Number	[]
efore Operation Checks	Calibration DateProbe S/N		
	Probe Nu	umber 1	
OR Trace 1 - Beginning Probe S	horted		
sor 16.331 m tance/Div25 m/div	ac	16.331 m	Tektronix 1502B JDF Date 1010ストラ
tical Scale 167 mp/div			··· Cable
se Filter 1 avs werac			Input Trace
	F		Stored Trace Difference Trace
	Fli.Lii		
ace Number 2 - Ending Probe S	Shorted		
or 16.451 m ance/Div25 m/div	ac	16.451 m	Tektronix 1502B TDR Date 1010293
ical Scale 182 m/div 0.99			Cable
e Filter 1 avs	<u> </u>		
	F		Input Trace Stored Trace
	¢		Difference Trace

Figure B-1. TDR Traces Obtained During Calibration

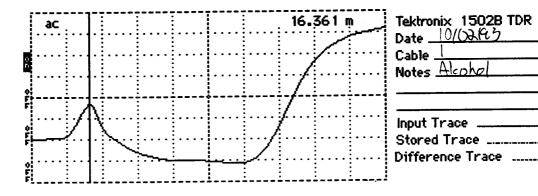
Probe Number 1 (cont.)

Frace Number 3 - Probe in Air

rsor	16.351 m
stance/Div	.25 m/div
rtical Scale	158 ms/div
	0.99
ise Filter	1 avs
wer	ac



Frace Number 4 - Probe in Alcohol



Trace Number 5 - Probe in Water

rsor	16.361 m
stance/Div	.25 m/div
rtical Scale	86.4 mp/div
	0.99
ise Filter	1 avs
wer	ac

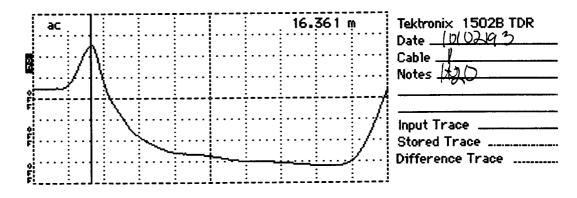
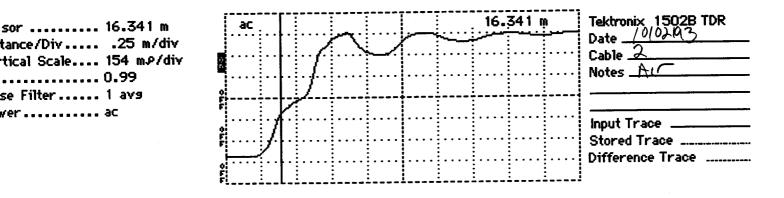


Figure B-1(cont.). TDR Traces Obtained During Calibration

LTPP Seasonal Mo	onitoring Study	State Code	[]		
TDR Probe C	alibration	Test Section Number	<u> </u>		
efore Operation Checks	Calibration DateProbe S/N				
	Probe N	umber 2			
DR Trace 1 - Beginning Probe	Shorted				
rsor	aC Pr	16.331 m	Tektronix 1502B TDR Date		
race Number 2 - Ending Probe	Shorted				
rsor	ac £	16.501 m	Tektronix 1502B TDR Date 10102000000000000000000000000000000000		

Figure B-1(cont.). TDR Traces Obtained During Calibration

race Number 3 - Probe in Air



race Number 4 - Probe in Alcohol

or 16.361 m	ac	·] [· :	:	 	16	. 36 1	Ŵ		Tektronix 1502B TDR
tance/Div25 m/div tical Scale81.6 m⊅/div							 	./	, 		• • • •	Cable Alcuhol
0.99 se Filter 1 avs	6	/	\		: · · · · · · · · · · · · · · · · · · ·	:	 	<i>}</i>				Tiones
ver ac		/	X	 \			 	<i>]</i>	 			Input Trace
	o ·				٠	<u> </u>	 ./		•	!		Difference Trace

race Number 5 - Probe in Water

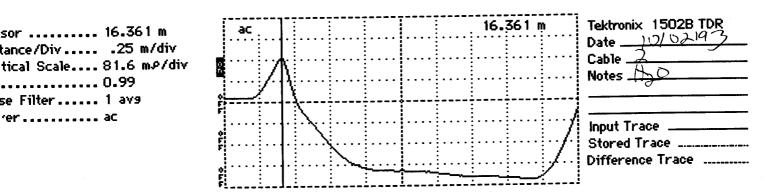


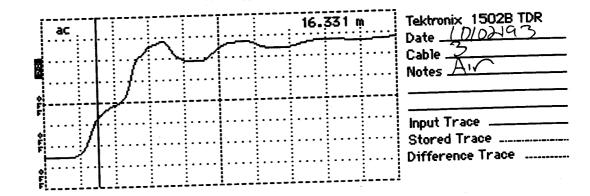
Figure B-1(cont.). TDR Traces Obtained During Calibration

	itoring Study	State Code	[]
TDR Probe Cal	libration	Test Section Number	
ore Operation Checks	Calibration DateProbe S/N		
	Probe N	umber 3	
R Trace 1 - Beginning Probe S	horted		
sor 16.311 m tance/Div25 m/div tical Scale 182 m/div 0.99	ac	16.311 m	Tektronix 1502B TDF Date 1002193 Cable 3 Notes Short CB
se Filter 1 avs	Ç F F		Input Trace Stored Trace
e Number 2 - Ending Probe S	Shorted		
	ac	16.451 m	Tektronix 1502B TDR Date
ce Number 2 - Ending Probe S er 16.451 m nce/Div 25 m/div cal Scale 182 m/div 0.99 Filter 1 avø r ac	;	16.451 m	Date <u>10102195</u> Cable <u>3</u>

Figure B-1(cont.). TDR Traces Obtained During Calibration

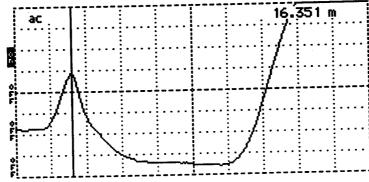
race Number 3 - Probe in Air

rsor	16.331 m
tance/Div	.25 m/div
rtical Scale	167 mp/div
ise Filter	
wer	ac



race Number 4 - Probe in Alcohol

or	.25 m/div 72.7 m/div 0.99
se Filter	1 avs



Tektronix 1502B TDR
Date _/0/02/93
Cable _5
Notes _Alcohol
Input Trace _____
Stored Trace _____

race Number 5 - Probe in Water

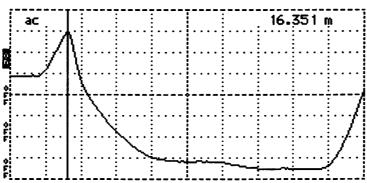


Figure B-1(cont.). TDR Traces Obtained During Calibration

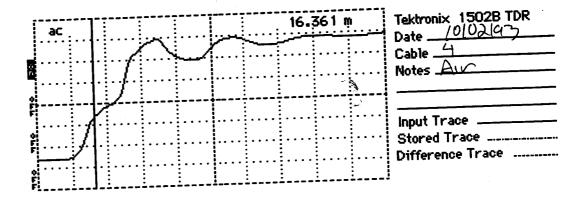
LTPP Seasonal Mo	nitoring Study	State Code	
TDR Probe Ca	alibration	Test Section Number	[]
Before Operation Checks	Calibration DateProbe S/N		
	Probe No	umber 4	•
DR Trace 1 - Beginning Probe S	Shorted		
rsor	ac	16.341 m	Tektronix 1502B TDR Date / 002193 Cable / 5hort /3 Input Trace Stored Trace Difference Trace
race Number 2 - Ending Probe	Shorted		
Cursor	ac F	16.501 m	Tektronix 1502B TCR Date

Figure B-1(cont.). TDR Traces Obtained During Calibration

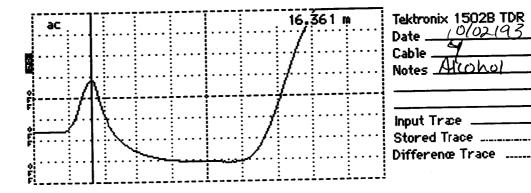
Probe Number 4 (cont.)

race Number 3 - Probe in Air

ursor	16.361 m
istance/Div	
ertical Scale	
P	
oise Filter	
ower	ac



race Number 4 - Probe in Alcohol



race Number 5 - Probe in Water

Cursor16.371 m
Distance/Div25 m/div
Vertical Scale81.6 m/div
VP0.99
Noise Filter1 avs
Powerac

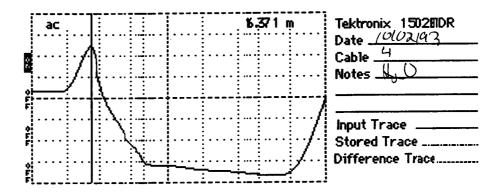


Figure B-1(cont.). TDR Traces Obtained During Calibration

LTPP Seasonal Monitoring Study		State Code	[]	
TDR Probe Ca	TDR Probe Calibration		[]	
fore Operation Checks	Calibration DateProbe S/N			
	Probe No	umber 5		
R Trace 1 - Beginning Probe	Shorted			
sor 16.381 m	ac	16.381 m	Tektronix 1502B TDR	
ance/Div25 m/div rical Scale 154 m.p/div 0.99 e Filter 1 avs			Date (0/024) Cable 5 Notes Shert CB	
erac	i A		input Trace	
	g .		··· Difference Trace	
ce Number 2 - Ending Probe	Shorted			
sor 16.501 m	ac	16.501 m	Tektronix 1502B TDR	
ance/Div25 m/div tical Scale188 m&/div 0.99			Date 1010243 Cable 5 Notes Shorted CN	
e Filter 1 avs erac				
	ę F		Input Trace	
	0 E		··· Difference Trace	

Figure B-1(cont.). TDR Traces Obtained During Calibration

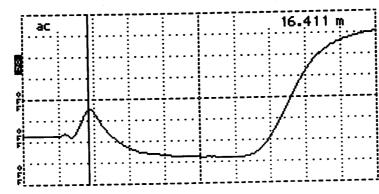
Probe Number 5 (cont.)

Trace Number 3 - Probe in Air

													·
ırsor 16.351 m		ac .								16.	351 r	n <u></u>	Tektronix 1502B TDR Date <u>/りのれる</u>
stance/Div25 m/div	j	:							<u> </u>				Cable 5
rtical Scale 163 mp/div			``		1				:	:			Notes In Air
0.99					1	:					, , , ,		
pise Filter 1 avs	F:	:		ريز:	/:	<u>:</u>	:		 :	 :	:	:	
owerac		:]	/ · · ·		· · · · ·				• • • •	: • • • •		Input Trace
	F		./										Stored Trace
		:	/			:	:	•	:	:		:	Difference Trace

Trace Number 4 - Probe in Alcohol

orstance/Div	.25 m/div
rtical Scale	122 m/div
ise Filter	0.99
wer	

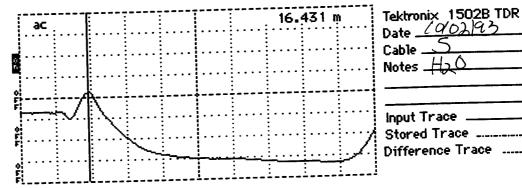


Notes In Alcoho Input Trace Stored Trace ____ Difference Trace

Tektronix 1502B TDR

Trace Number 5 - Probe in Water

. 16.431 m istance/Div......25 m/div



Cable . Notes -Input Trace Stored Trace _____ Difference Trace

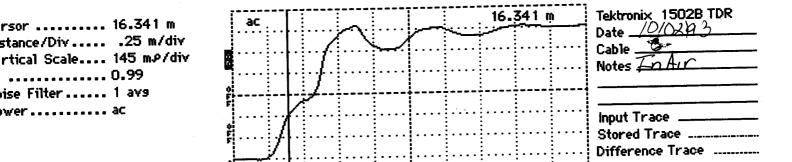
Figure B-1(cont.). TDR Traces Obtained During Calibration

LTPP Seasonal Mo	nitoring Study	State Code	ll	
TDR Probe Ca	alibration	Test Section Number	[]	
Before Operation Checks	- Calibration Date - Probe S/N			
	Probe Nur	nber 6		
TDR Trace 1 - Beginning Probe	Shorted			
sor 16.321 m tance/Div 25 m/div tical Scale 200 m.p/div 0.99 se Filter 1 avs ver ac	ac our	16.321 m	Tektronix 1502B TDR Date/2//2/5 Cable _G Notes Short @ CB Input Trace Stored Trace Difference Trace	
Trace Number 2 - Ending Probe	Shorted	16.331 m	Tektronix 1502B TDR	
tance/Div18331 m tance/Div25 m/div rtical Scale182 m/div 0.99			Date 10/02/93 Cable 6 Notes Short @ end	
ise Filter 1 avs werac	Ç.		Input Trace	
	o F		Stored Trace	

Figure B-1(cont.). TDR Traces Obtained During Calibration

Probe Number 6 (cont.)

Trace Number 3 - Probe in Air



Trace Number 4 - Probe in Alcohol

or 16.361 m	ac .	1	- <u>-</u> -	<u>-</u>				16	.361	W	 Tektronix 1502B TDR
ance/Div25 m/div ertical Scale 129 m/div										سنبي	 Date 10102195 Cable B Notes IN Alcohol
oise Filter 1 avs	о F]				 		/	<u>.</u>	. <u>.</u> :-	
owerac	<u></u>		1	· · · · ·	• • • •	 	<i>J</i>	<i>y</i>			Input Trace Stored Trace
	o F				سننر	 	جمبر <u> </u>			! .	 Difference Trace

Trace Number 5 - Probe in Water

Tektronix 1502B TDR 16.361 m istance/Div..... .25 m/div Date. Cable ertical Scale.... 126 mp/div Notes <u>→</u>△ Input Trace Stored Trace . Difference Trace

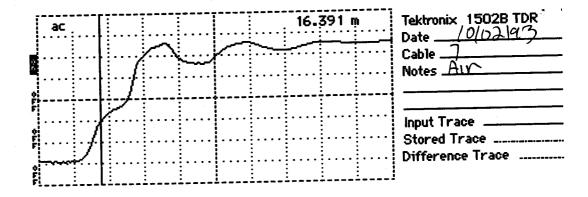
Figure B-1(cont.). TDR Traces Obtained During Calibration

LTPP Seasonal Mo	onitoring Study	State Code	[]
TDR Probe C	alibration	Test Section Number	[]
fore Operation Checks	Calibration DateProbe S/N		
	Probe No	umber 7	
R Trace 1 - Beginning Probe	Shorted		
or 16.391 m	ac		Tektronix 1502B TDR
nce/Div25 m/div	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \		Date
cal Scale 163 m₽/div 0.99	I		Cable <u>Short CB</u>
Filter 1 avs			Notes 2707
rac	'		
			Input Trace Stored Trace
		·	Difference Trace
	FliLii		d
ace Number 2 - Ending Probe	Shorted		
200 (1011120) 2 21121119 1 1 2 2			
16 E01 m	ac 🛴	16.501 m	Tektronix 1502B TDR
or 16.501 m ance/Div25 m/div			Date 10/02/97
ical Scale 163 mø/div			Cable 7
0.99 e Filter 1 avs			· Notes Shorten
e riiter i avs erac	o F		
	0 F		Input Trace
			Stored Trace Difference Trace

Figure B-1(cont.). TDR Traces Obtained During Calibration

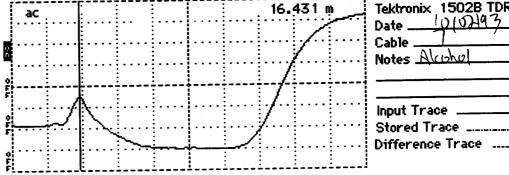
race Number 3 - Probe in Air

rsor	16.391 m
tance/Div	
rtical Scale	163 mp/div
ise Filter	
wer	ac



Frace Number 4 - Probe in Alcohol

yrsor 16.431 m istance/Div......25 m/div ertical Scale.... 115 ms/div



Tektronix 1502B TDR Date. Cable . Notes Alushu Input Trace Stored Trace ____

Trace Number 5 - Probe in Water

... 16.471 m stance/Div..... .25 m/div rtical Scale....83.9 mp/div

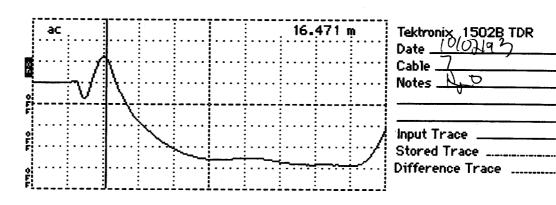


Figure B-1(cont.). TDR Traces Obtained During Calibration

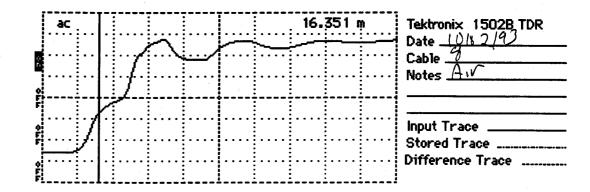
LTPP Seasonal M	onitoring Study	State Code	[]
TDR Probe (Calibration	Test Section Number	<u></u>
Before Operation Checks	- Calibration Date - Probe S/N		
	Probe Nu	ımber 8	
TDR Trace 1 - Beginning Probe	Shorted		
rsor 16.331 m stance/Div 25 m/div rtical Scale 163 mø/div 0.99 ise Filter 1 avø wer ac	aC g	16.331 m	Tektronix 1502B TDR Date 1010 197 Cable 8 Notes Sort 15 Input Trace Stored Trace Difference Trace
Trace Number 2 - Ending Probe	e Shorted		
ursor	ac F	16.501 m	Tektronix 1502B TDR Date /0/02/93 Cable 8 Notes Shortend Input Trace Stored Trace Difference Trace

Figure B-1(cont.). TDR Traces Obtained During Calibration

Probe Number 8 (cont.)

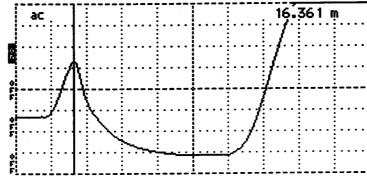
Frace Number 3 - Probe in Air

ursor	.16.351 m
istance/Div	25 m/div
ertical Scale	. 167 ms/div
·	.0.99
oise Filter	.1 avs
wer	. ac



Trace Number 4 - Probe in Alcohol

)r	16.361 m
tance/Div	.25 m/div
tical Scale	72.7 mp/div.
	0.99
se Filter	1 avs
ver	ac



Tektronix 1502B TDR
Date 40/02/93
Cable 8
Notes Alcohol
Input Trace _____
Stored Trace _____
Difference Trace _____

Trace Number 5 - Probe in Water

or 16.361 m nce/Div 25 m/div cal Scale..... 72.7 m₽/div 0.99 o Filter...... 1 avs

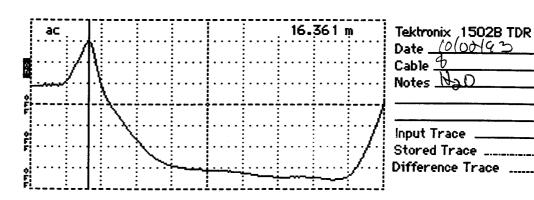
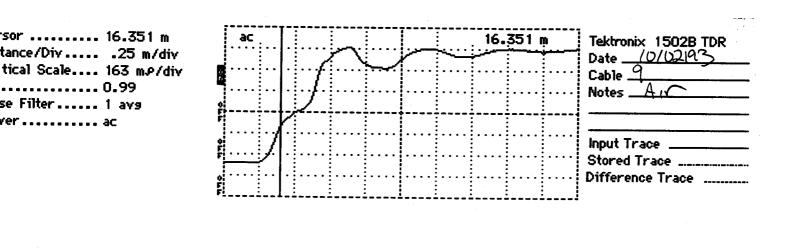


Figure B-1(cont.). TDR Traces Obtained During Calibration

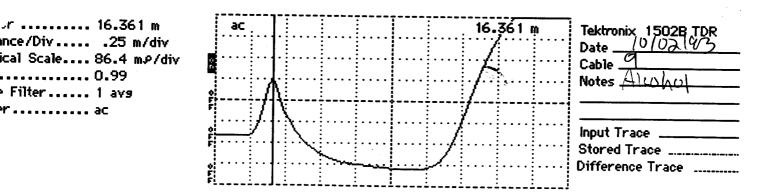
	onitoring Study	State Code	[]
TDR Probe C	Calibration	Test Section Number	()
ore Operation Checks	Calibration DateProbe S/N		
	Probe N	umber 9	
Trace 1 - Beginning Probe	Shorted		
r 16.351 m	[ac	16.351 m	Telebracia 45000 TDS
nce/Div25 m/div cal Scale154 m/div 0.99	ac	10.331 111	Tektronix, 1502B TDR Date 1010193 Cable 9 Notes 21011 CB
Filter 1 avs rac	ř ř		Input Trace
	F		Difference Trace
ce Number 2 - Ending Probe	Shorted		
ce Number 2 - Ending Probe	Shorted		
ce Number 2 - Ending Probe	Shorted		
16.481 m	Shorted	16.481 m	Tektronix 1502B TDR Date <u>(ののみ</u> 名子
· 16.481 m ce/Div25 m/div	iiiii	16.481 m	Tektronix 1502B TDR Date
·	iiiii	16.481 m	Date <u>/ 0/02/97</u> Cable <u>9</u>
·	iiiii	16.481 m	Date 1010293 Cable 9 Notes Shorton Input Trace
ce Number 2 - Ending Probe	iiiii	16.481 m	Date 101029 Cable 9 Notes Shorton

Figure B-1(cont.). TDR Traces Obtained During Calibration

ace Number 3 - Probe in Air



race Number 4 - Probe in Alcohol



race Number 5 - Probe in Water

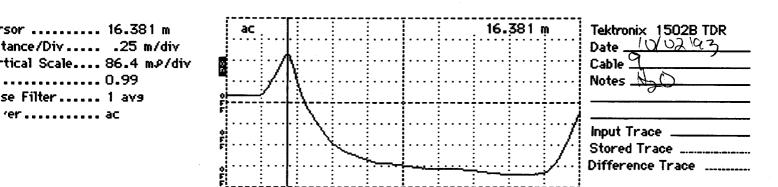
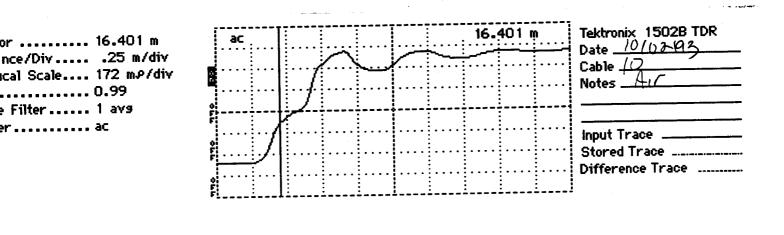


Figure B-1(cont.). TDR Traces Obtained During Calibration

LTPP Seasonal Mo	onitoring Study	State Code	[]
TDR Probe C	Calibration	Test Section Number	[]
efore Operation Checks	Calibration DateProbe S/N		
	Probe No	umber 10	
OR Trace 1 - Beginning Probe	Shorted		
sor 16.421 m ance/Div 25 m/div tical Scale 158 m/div 0.99 se Filter 1 avs rer ac	ac out	16.421 m	Tektronix 1502B TDR Date / b/nal 43 Cable / b Notes Short CB Input Trace Stored Trace Difference Trace
ace Number 2 - Ending Probe	e Shorted		
sor 16.491 m ance/Div 25 m/div tical Scale 158 m/div 0.99 se Filter 1 avs	ac ¢	16.491 m	Tektronix 1502B TDR Date //0/02/93 Cable //O Notes // Cable //O Input Trace

Figure B-1(cont.). TDR Traces Obtained During Calibration

race Number 3 - Probe in Air



Frace Number 4 - Probe in Alcohol

or 16.411 m	ac .]: 	: :	T		16	411 n	<u> </u>	Tektronix 1502B TDR
stance/Div25 m/div rtical Scale126 m₽/div 0.99									Date 1010213 Cable 10 Notes Alcohol
ise Filter 1 avs werac									Input Trace Stored Trace
	Ç.				<u>:</u>		<u>:</u>	<u> </u>	

Frace Number 5 - Probe in Water

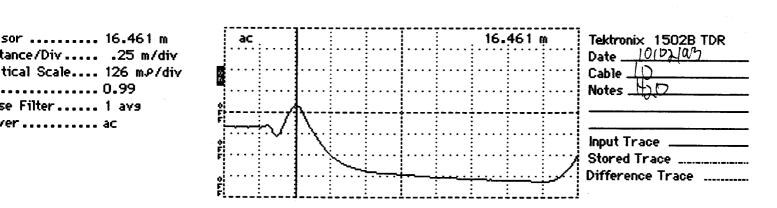


Figure B-1(cont.). TDR Traces Obtained During Calibration

APPENDIX C

Supporting Instrumentation Installation Information

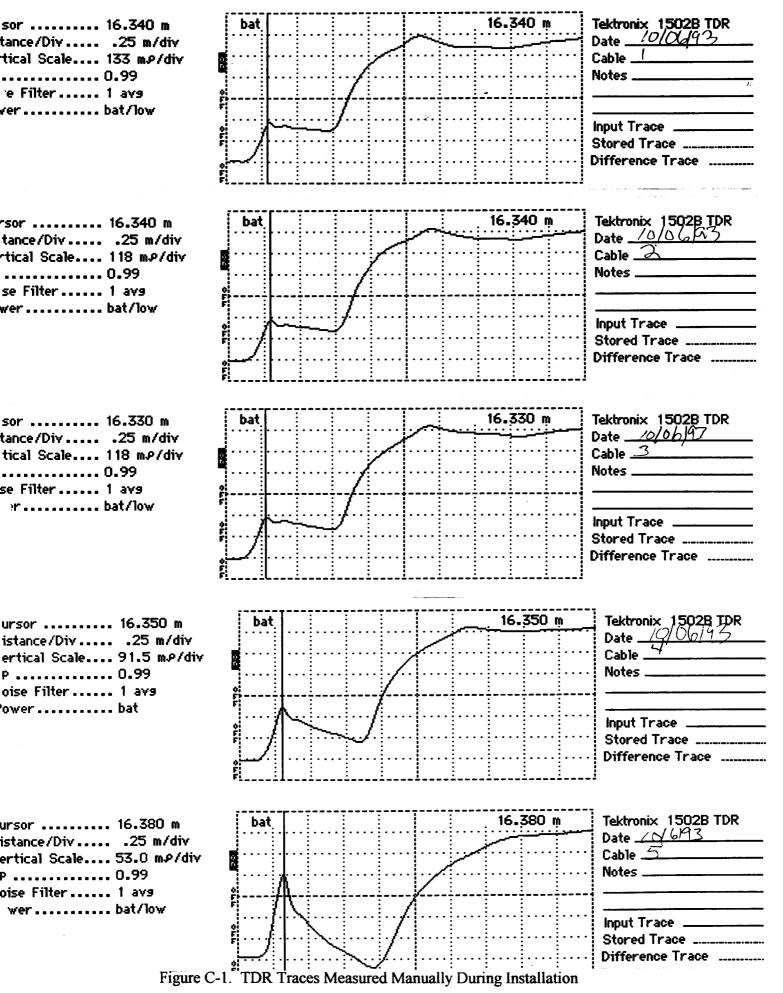
Appendix C contains the following supporting information:

Figure C-1 TDR Traces Measured Manually During Installation

Table C-1 TDR Moisture Content During Installation

Table C-2 Field Measured Moisture Content During Installation

Laboratory Moisture Samples' Results as Received from the State



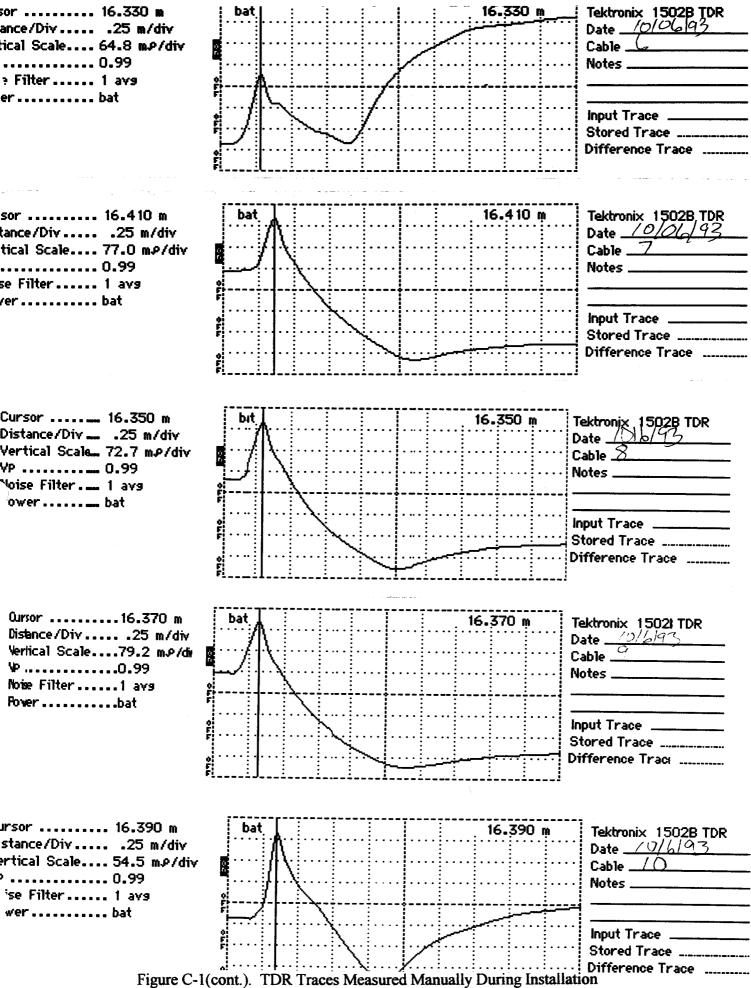


Table C-1. TDR Moisture Content

TDR No.	TDR Length (m)	Dielectric Constant (ε)	Volumetric Moisture Content (%)	In-Situ Dry Density (kg/m³)	Gravimetric Moisture Content (%)
50A01	0.450	5.01	8.06	2.06	3.91
50A02	0.510	6.44	11.40	1.99	5.73
50A03	0.510	6.44	11.40	1.99	5.73
50A04	0.580	8.33	15.54	1.99	7.81
50A05	0.660	10.79	20.44	2.09	9.78
50A06	0.670	11.11	21.06	2.09	10.08
50A07	0.950	22.35	37.51	2.09	17.95
50A08	0.880	19.17	33.69	2.09	16.12
50A09	1.070	28.35	43.36	2.09	20.74
50A10	0.760	14.30	26.61	2.09	12.73

Table C-2. Field Measured Moisture Content

Study	State Cod		[50]	
8	Test Secti	on Number		[1002]
Probe	Probe 2	Probe 3	Probe 4	Probe 5
367.4	271.2	218.5		241.0
364.2	268.2	215.7	*	228.1
120.6	120.6	120.6	*	120.9
243.6	147.6	95 1	*	107.2
246.8	150.6	97.9	*	120.1
				T
3.2	3.0	2.8	*	12.9
1.31	2.03	2.94	*	12.03
Prohe	Probe	Probe	Probe	Probe
6	7	8	9	10
	_			
				227.5
216.8	193.9	206.9	210.4	206.2
120.6	120.2	120.6	121.4	120.6
96.2	73.7	86.3	89.0	85.6
108.7	87.9	98.0	111.6	106.9
12.5	14.2	11.7	22.6	21.3
12.99	19.27	13.56	25.39	24.88
	1 367.4 364.2 120.6 243.6 246.8 3.2 1.31 Probe 6 229.3 216.8 120.6 96.2 108.7	Probe Probe 1 2 367.4 271.2 364.2 268.2 120.6 120.6 243.6 147.6 246.8 150.6 3.2 3.0 Probe 6 7 229.3 208.1 216.8 193.9 120.6 120.2 96.2 73.7 108.7 87.9 12.5 14.2	Probe Probe Probe 3 367.4 271.2 218.5 364.2 268.2 215.7 120.6 120.6 120.6 243.6 147.6 95.1 246.8 150.6 97.9 3.2 3.0 2.8 Probe Probe Probe 6 7 8 229.3 208.1 218.6 216.8 193.9 206.9 120.6 120.2 120.6 96.2 73.7 86.3 108.7 87.9 98.0 12.5 14.2 11.7	Probe Probe Probe Probe Probe 4 367.4 271.2 218.5 * 364.2 268.2 215.7 * 120.6 120.6 120.6 * 243.6 147.6 95.1 * 246.8 150.6 97.9 * 3.2 3.0 2.8 * 1.31 2.03 2.94 * Probe 6 7 8 9 229.3 208.1 218.6 233.0 216.8 193.9 206.9 210.4 120.6 120.2 120.6 121.4 96.2 73.7 86.3 89.0 108.7 87.9 98.0 111.6 12.5 14.2 11.7 22.6

^{*} Note: No field moisture for probe #4



STATE OF VERMONT

AGENCY OF TRANSPORTATION

133 State Street, Administration Building

Montpelier, Vermont 05633-



October 27, 1993

Mr. Brandt Henderson Pavement Management Systems 415 Lawrence Bell Drive Amherst, NY 15221

RE: Seasonal testing - Sample moisture content

Dear Mr. Henderson,

The following are the results of the moisture content analysis run on the soil samples taken from the New Haven seasonal test installation on 10/06/1993.

Sample	% moisture	Sample	% moisture
1	2.20%	6	13.79%
2	1.63%	7	22.31%
3	1.48%	8	19.70%
4	N/A	9	25.45%
5	11.53%	10	23.67%

Sincerely yours

Chris Benda

APPENDIX D

Initial Data Collection

Appendix D contains the following supporting information:

Figure D-1	Initial First Set of TDR Traces Measured with the Mobile Unit
Figure D-2	Initial Second Set of TDR Traces Measured with the Mobile Unit
Figure D-3	Voltages Measured Using the Mobile System
Figure D-4	Manually Collected Contact Resistance
Figure D-5	Manually Collected Four-Point Resistivity
Table D-1	Contact Resistance After Installation
Table D-2	Four-Point Resistivity After Installation
Table D-3	Uniformity Survey Results Before and After Installation
Figure D-6	Deflection Profiles from FWDCHECK (Test Date and Time October 06, 1993 @ 0943)
Table D-4	Subgrade Modulus and Structural Number from FWDCHECK (Test Date and Time October 06, 1993 @ 0943)
Figure D-7	Deflection Profiles from FWDCHECK (Test Date and Time October 07, 1993 @ 0952)
Table D-5	Subgrade Modulus and Structural Number from FWDCHECK (Test Date and Time October 07, 1993 @ 0952)
Figure D-8	Deflection Profiles from FWDCHECK (Test Date and Time October 07, 1993 @ 1144)
Table D-6	Subgrade Modulus and Structural Number from FWDCHECK (Test Date and Time October 07, 1993 @ 1144)
Figure D-9	Deflection Profiles from FWDCHECK (Test Date and Time October 07, 1993 @ 1347)
Table D-7	Subgrade Modulus and Structural Number from FWDCHECK (Test Date and Time October 07, 1993 @ 1347)
Table D-8	Surface Elevation Measurements

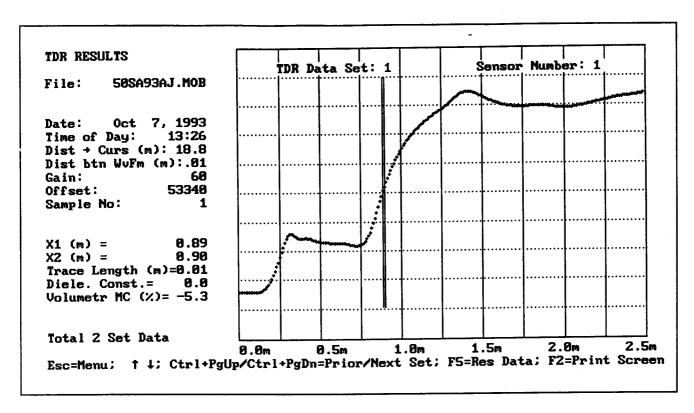


Figure D-1. Initial First Set of TDR Traces Measured with the Mobile Unit

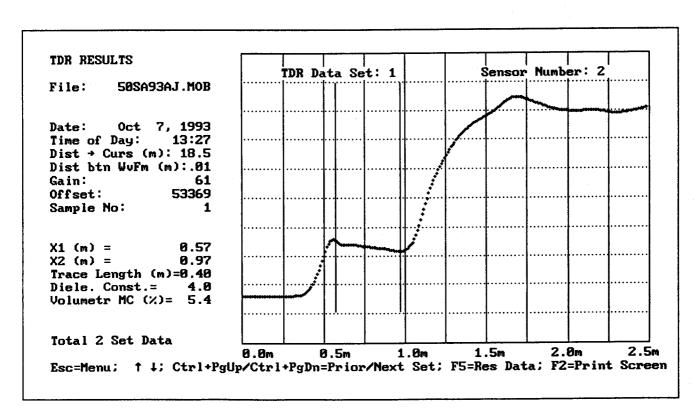


Figure D-1(cont.). Initial First Set of TDR Traces Measured with the Mobile Unit

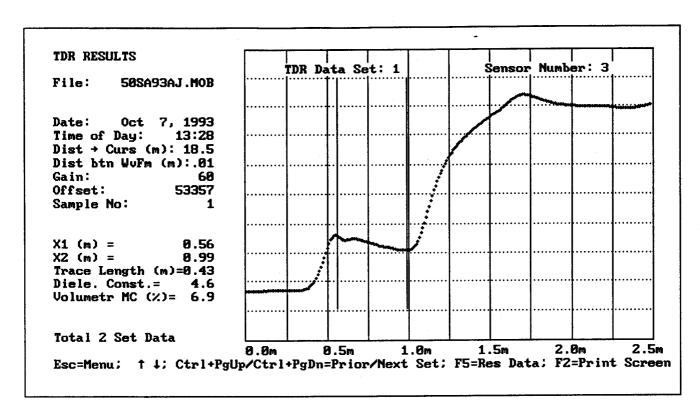


Figure D-1(cont.). Initial First Set of TDR Traces Measured with the Mobile Unit

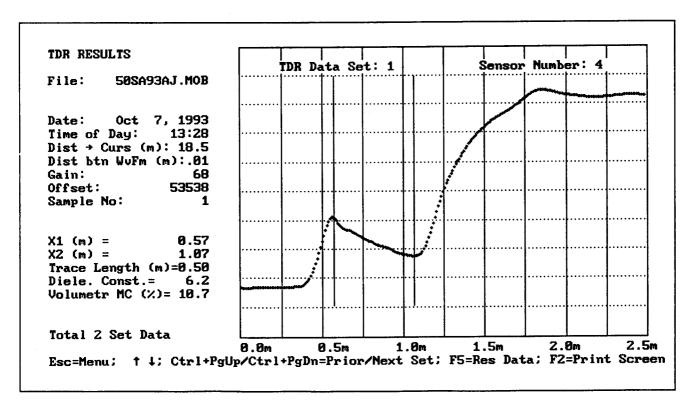


Figure D-1(cont.). Initial First Set of TDR Traces Measured with the Mobile Unit

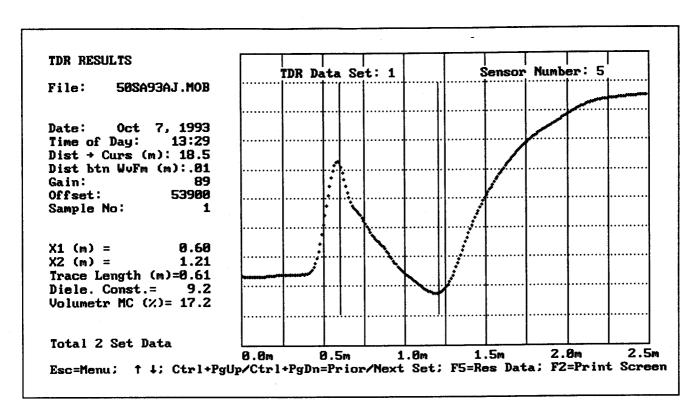


Figure D-1(cont.). Initial First Set of TDR Traces Measured with the Mobile Unit

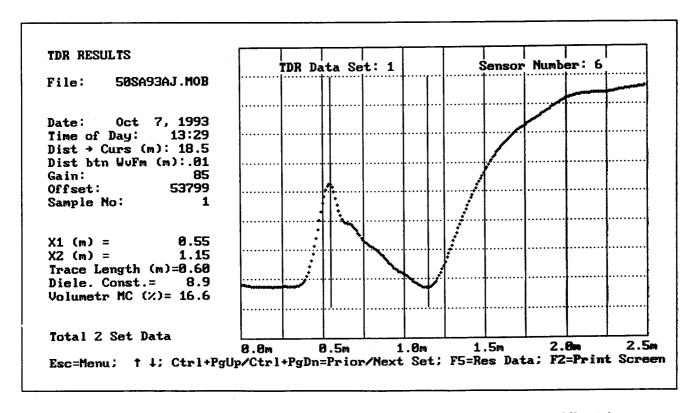


Figure D-1(cont.). Initial First Set of TDR Traces Measured with the Mobile Unit

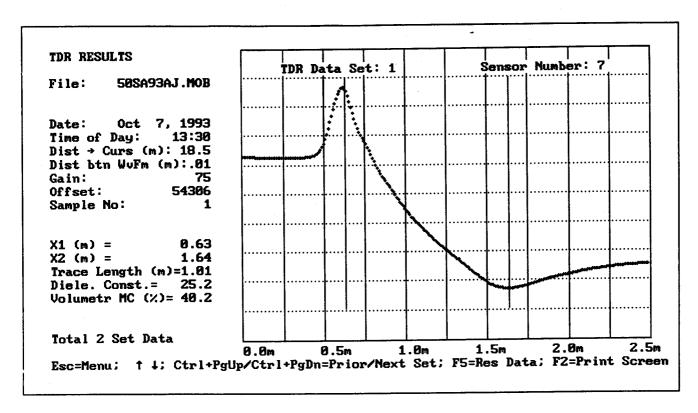


Figure D-1(cont.). Initial First Set of TDR Traces Measured with the Mobile Unit

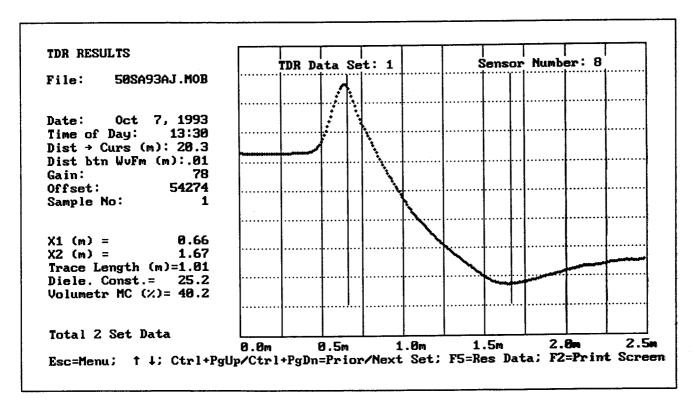


Figure D-1(cont.). Initial First Set of TDR Traces Measured with the Mobile Unit

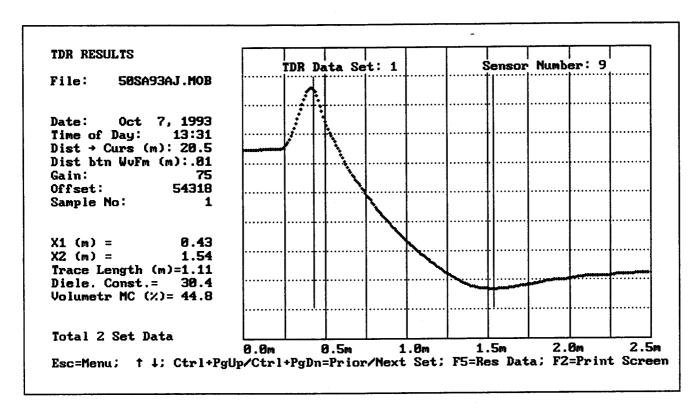


Figure D-1(cont.). Initial First Set of TDR Traces Measured with the Mobile Unit

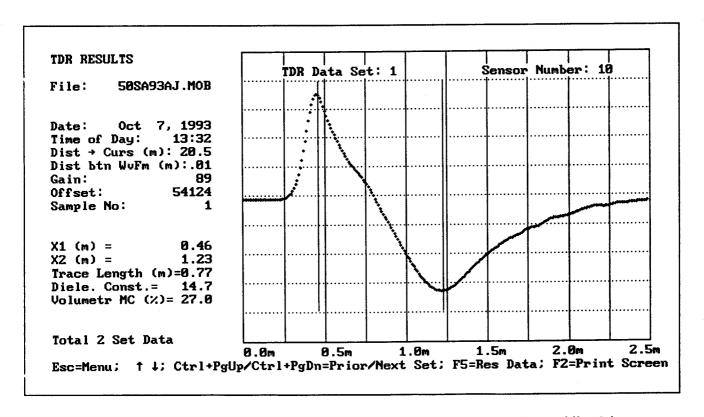


Figure D-1(cont.). Initial First Set of TDR Traces Measured with the Mobile Unit

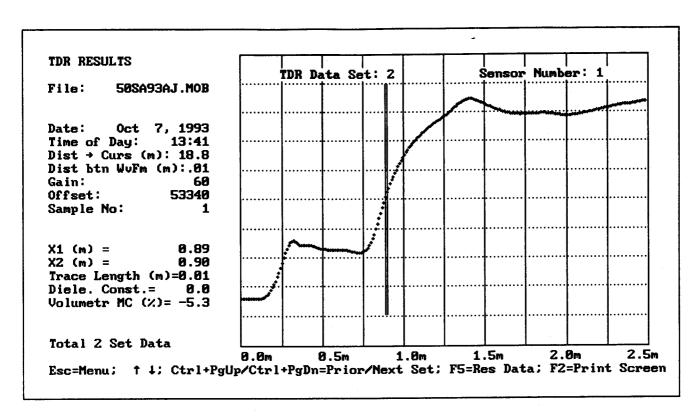


Figure D-2. Initial Second Set of TDR Traces Measured with the Mobile Unit

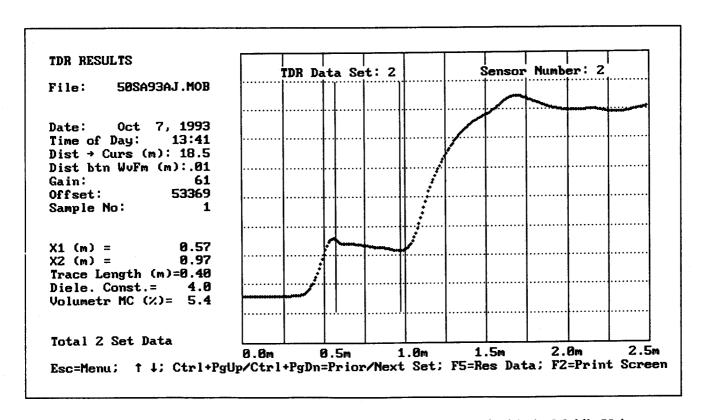


Figure D-2(cont.). Initial Second Set of TDR Traces Measured with the Mobile Unit

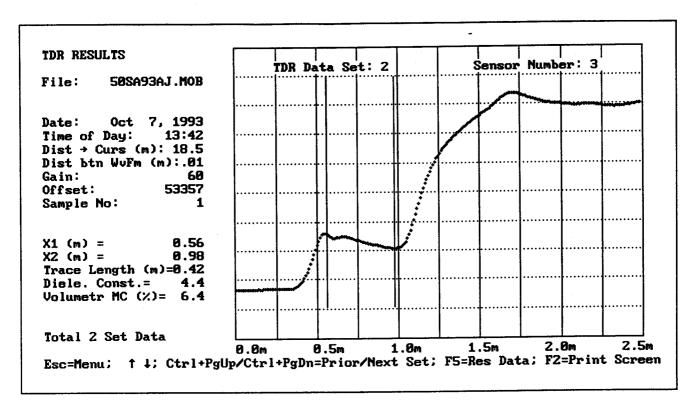


Figure D-2(cont.). Initial Second Set of TDR Traces Measured with the Mobile Unit

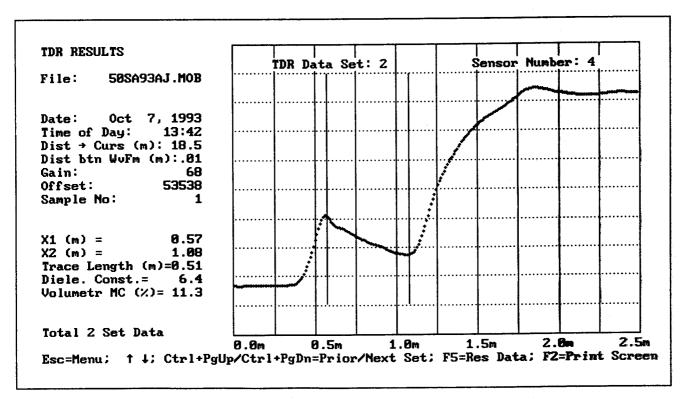


Figure D-2(cont.). Initial Second Set of TDR Traces Measured with the Mobile Unit

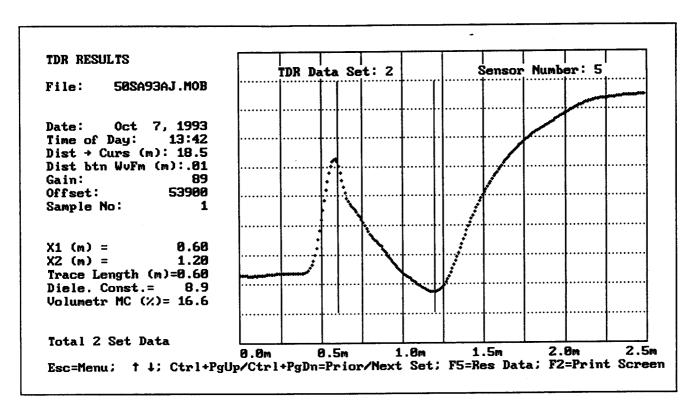


Figure D-2(cont.). Initial Second Set of TDR Traces Measured with the Mobile Unit

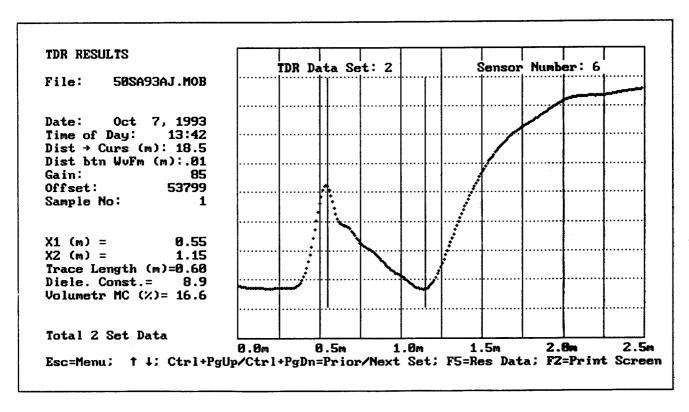


Figure D-2(cont.). Initial Second Set of TDR Traces Measured with the Mobile Unit

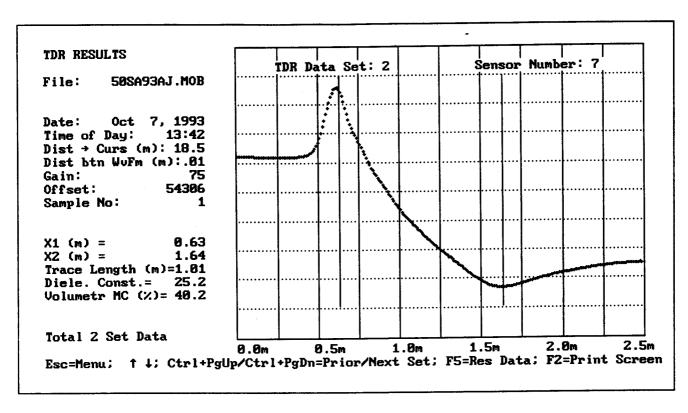


Figure D-2(cont.). Initial Second Set of TDR Traces Measured with the Mobile Unit

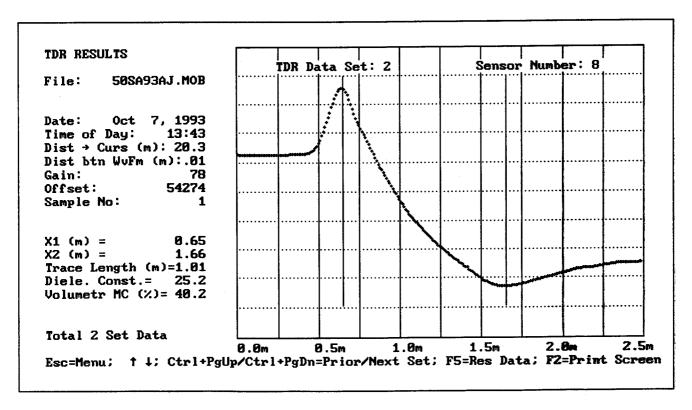


Figure D-2(cont.). Initial Second Set of TDR Traces Measured with the Mobile Unit

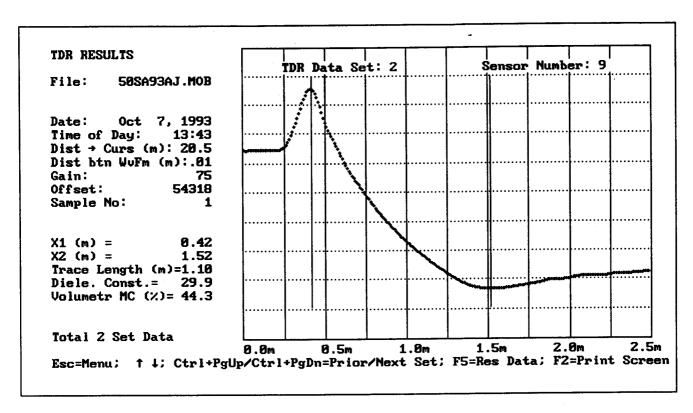


Figure D-2(cont.). Initial Second Set of TDR Traces Measured with the Mobile Unit

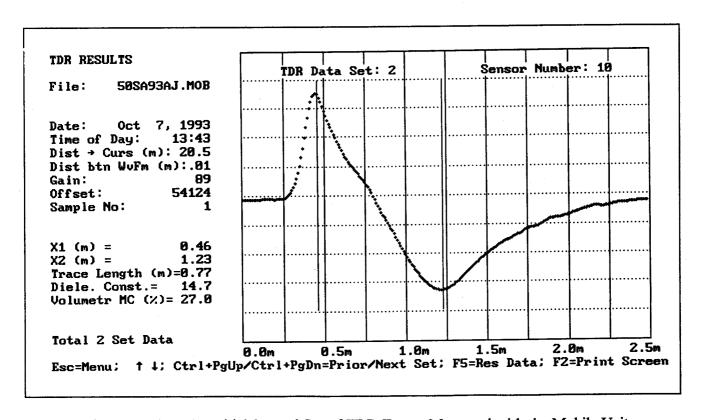


Figure D-2(cont.). Initial Second Set of TDR Traces Measured with the Mobile Unit

Voltage (millivolt) 50 150 200 250 300 350 0 100 0 Depth (m) from pavement 0.2 0.4 0.6 0.8 1.2 1.4 1.6 1.8

2 2.2

SECTION 501002

Figure D-3. Voltages Measured Using the Mobile System During Initial Data Collection, October 7, 1993

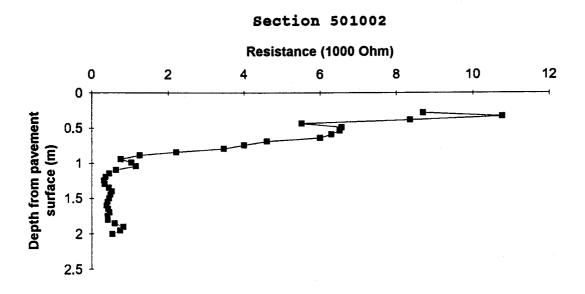


Figure D-4. Manually Collected Contact Resistance During Initial Data Collection, October 7, 1993

Section 501002 Resistance (10 Ohm) 5 45 0 10 15 20 25 30 35 40 0 0.2 Depth from pavement 0.4 0.6 surface (m) 8.0 1.2 1.4 1.6 1.8 2

Figure D-5. Manually Collected Four Point Resistivity During Initial Data Collection, October 7, 1993

Table D-1. Contact Resistance After Installation

LTPP Seasonal Monitoring Study	State Code	[50]
Data Sheet R1		
Contact Resistance Measurements	Test Section Number	[1002]

1. Date (Month-Day-Year)	[10-07-93]
2. Time Measurements Began (Military)	[1125]
3. Comments	After Installation * Note: Known Resistors

Test Position	Conne	ections	Voltage	(ACV)	Curren	t (ACA)	notes
	I	I V	Range	Reading	Range Setting	Reading	
	V		Setting	2150		24.7	
1	1	2	mV	215.0	uA	18.9	
2	3	2	mV_	203.5	uA.		
3	3	4	mV	201.6	uA	24.1	
4	5 ,	4	mV	193.9	uA_	35.1	
5	5	6	mV	196.4	uA	29.9	
6	7	6	mV	194.8	uA_	29.9	
7	7	8	mV	168.1	uA	26.7	
8	9	8	mV	167.5	uA	27.9	
9	9	10	mV	163.3	uA	35.5	
10	11	10	mV	160.9	uA	40.2	ļ
11	11	12	mV	157.6	uA	45.5	
12	13	12	mV	144.6	uA	65.4	
13	13	14	mV	123.6	uA	97.9	
14	15	14	mV	100.7	uA	131.9	
15	15	16	mV	115.6	uA	110.8	
16	17	16	mV	119.6	uA	102.9	
17	17	18	mV	92.1	uA	144.3	
18	19	18	mV	76.7	uA	166.3	
19	19	20	mV	66.2	uA	181.3	
20	21	20	mV	60.4	uA	188.5	
21	21	22	mV	62.9	uA	185.2	
22	23	22	mV	76.0	uA	167.7	
23	23	24	mV	81.9	· uA	156.8	
24	25	24	mV	78.9	uA	161.0	
25	25	26	mV	77.0	uA	166.9	
26	27	26	mV	72.3	uA	172.6	
27	27	28	mV	70.5	uA	180.9	
28	29	28	mV	75.3	uA	174.2	
29	29	30	mV	79.3	uA	170.0	
30	31	30	mV	73.9	uA	176.0	
31	31	32	mV	73.9	uA	173.2	
32	33	32	mV	89.9	uA	148.8	
33	33	34	mV	104.6	uA	126.0	
34	35	34	mV	99.1	uA	133.1	
35	35	36	mV	83.2	uA	154.1	
36 *	37	38	mV		uA		
37 *	38	39	mV		uA		
38 *	39	40	mV		uA		
Preparer:		chael Zawisa		nployer:		MSL	

Table D-2. Four-Point Resistivity After Installation

ra	LTPP Seasonal Monitoring Study						State Code [5				
L.	Data Sheet R2						State Code [5 0]				
<u> </u>						The Continuation [1002]					
Fou	r-Point	Resistiv	vity Me	asureme	ents	Test Section Number [1 0 0 2]					
1. Date	(Mont	h Day	Venr)	*				Γ10	-07-931		
1. Date	(IMOIII)	in-Day-	i ear)					[10	-07-23]		
								· · · · · · · · · · · · · · · · · · ·	F11457		
2. Time	e measu	irement	s Begar	ı (Milita	ıry)				[1145]		
3. Con	nments							After Inst	allation		
f T		0			V-1	tana (ACV)	Curron	t (ACA)	r i		
T	 1	Conne		7		tage (ACV)		Reading	Notes		
Test Position	I_1	v ₁	v_2	I ₂	Range Setting		Range Setting	Reading	14000		
1	1	2	3	4	mV	10.4	uA	26.8	Ì		
2	2	3	4	5	mV	7.1	uA	18.5			
3	3	4	5	6	mV	6.3	uA	16.7			
4	4	5	6	7	mV	7.5	uA	27.5			
5	5	6	7	8	mV	7.1	uA	24.0			
6	6	7	8	9	mV	8.5	uA	24.2			
7	7	8	9	10	mV	8.5	uA	30.7			
8	8	9	10	11	mV	9.8	uA	27.2			
9	9	10	11	12	mV	8.1	uA	35.6			
10	10	11	12	13	mV	8.5	uA	49.1			
11	11	12	13	14	mV	5.3	uA	54.2			
12	12	13	14	15	mV	6.6	uA	71.6			
13	13	14	15	16	mV	3.5	uA	80.6	 		
14	14	15	16	17	mV	8.5	uA	114.2	-		
15	15	16	17	18	mV	5.6	uA	146.0			
16	16	17	18	19	mV	2.1	uA	109.0			
17	17	18	19	20	mV	2.8	uA	148.4			
18	18	19	20	21	mV	2.9	uA	163.5			
19	19	20	21	22	mV	2.5	uA	170.0			
20	20	21	22	23	mV	3.2	uA uA	162.1			
21	21	22	23	24	mV	2.5	uA	165.8			
22	22	23	24	25	mV	3.2	uA	163.8			
23	23	24	25	26	mV	2.2	uA	156.6	 		
24	24	25	26	27	mV	2.6	uA	160.8			
25	25	26	27	28	mV	2.2	uA	166.2			
26	26	27	28	29	mV	2.2	uA	160.7			
27	27	28	29	30	mV	2.1	uA	168.9			
28	28	29	30	31	mV	2.3	uA	172.1	1		
29	29	30	31	32	mV	1.9	uA	160.5			
30	30	31	32	33	mV	1.8	uA	144.8			
31	31	32	33	34	mV	1.8	uA	138.0	 		
32	32	33	34	35	mV	2.1	uA	152.8	1		
33	33	34	35	36	mV	2.2	uA	139.6			
Preparer			MZ			Employer	PM	ISL			

Table D-3. Uniformity Survey Results Before and After Installation

Seasonal Uniformity Survey	Falling Weight Deflectometer
Site Number: 331001	Data Collection and
Date Surveyed: October 06-October 07, 1993	Processing Summary

Date Surveye	a. Octobe	1 00-0010	JCI 07, 17	7.5	1100033111	5 Dullillian	<i>y</i>		
Section Interval (ft)	М	ean Deflect for HT : Corre	2 (mils)	es				Mean Temp D1 (F)	
	Sensor 1	Sensor 1 std dev	Sensor 7	Sensor 7 std dev	Subg modulus (psi)	Subg modulus std dev	Effective SN	SN std dev	
-20 to 200 Oct 06 @ 0943	6.19	0.53	1.52	0.15	24934	2282	9.75	0.32	**
-20 to 175* Oct 07 @ 0952	6.69	0.52	1.46	0.15	25547	1943	9.36	0.35	57.9
-20 to 200 Oct 07 @ 1144	7.42	0.66	1.49	0.17	24830	2077	8.95	0.33	67.9
-20 to 200	7.87	0.68	1.56	0.11	24094	1480	8.74	0.35	75.4

* Note: Station 200 not tested

Oct 07
<a>@ 1347

** Note: No temperatures recorded on October 06, 1993

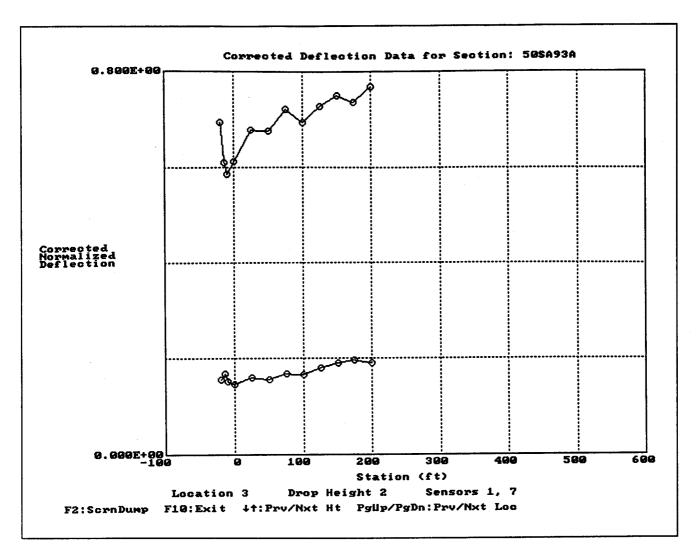


Figure D-6. Deflection Profiles from FWDCHECK (Test Date and Time October 06, 1993 @ 0943)

Table D-4. Subgrade Modulus and Structural Number from FWDCHECK (Test Date and Time October 06, 1993 @ 0943)

Flexible Pavement Thickness Statistics - 50SA93A - Drop Height 2						
Subsection	Station	Station Subgrade Modulus				
1	-20	26632	9.60			
	-15	24975	10.30			
	-10	27301	10.35			
	0	28345	10.10			
	25	26215	9.70			
	50	26816	9.70			
	75	24949	9.50			
	100	25219	9.65			
	125	23278	9.55			
	150	21926	9.50			
	175	21472	9.60			
	200	22084	9.40			
Subsection 1	Overall Mean	24934	9.75			
	Standard Deviation	2282	0.32			
	Coeff of Variation	9.15%	3.29%			

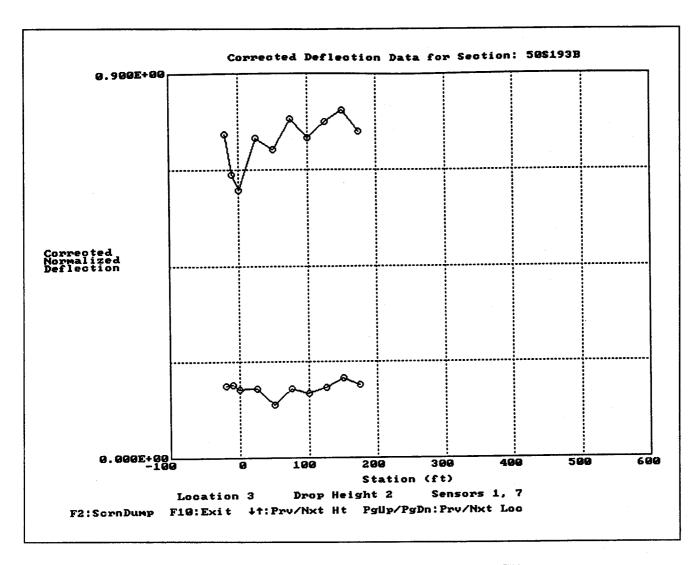


Figure D-7. Deflection Profiles from FWDCHECK (Test Date and Time October 07, 1993 @ 0952)

Table D-5. Subgrade Modulus and Structural Number from FWDCHECK (Test Date and Time October 07, 1993 @ 0952)

Flexible Pavement Thickness Statistics - 50S193B - Drop Height 2						
Subsection	Station	Subgrade Modulus	Effective SN			
1	-20	24768	9.30			
	-10	24389	9.90			
	0	26199	10.10			
	25	25686	9.30			
	50	30061	9.25			
	75	25817	9.05			
	100	26240	9.25			
	125	25473	9.10			
	150	22521	9.10			
	175	24319	9.25			
	200*					
Subsection 1	Overall Mean	25547	9.36			
	Standard Deviation	1943	0.35			
	Coeff of Variation	7.61%	3.76%			

* Note: Station 200 not tested

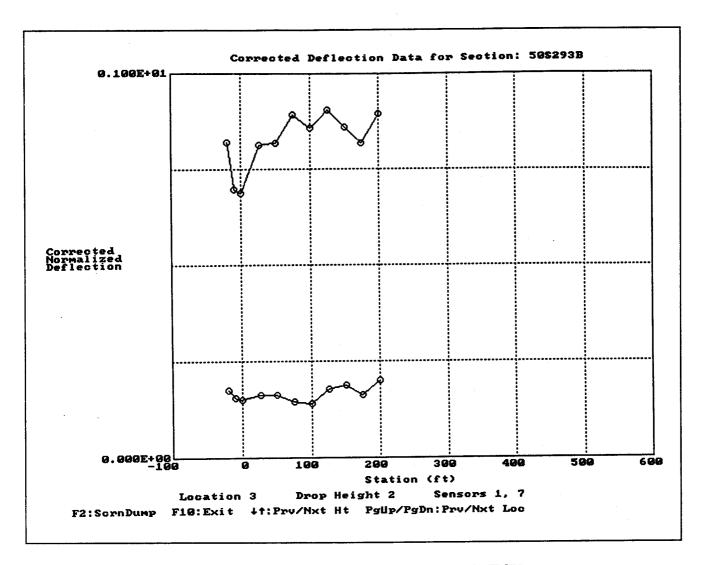


Figure D-8. Deflection Profiles from FWDCHECK (Test Date and Time October 07, 1993 @ 1144)

Table D-6. Subgrade Modulus and Structural Number from FWDCHECK (Test Date and Time October 07, 1993 @ 1144)

Flexible I	Flexible Pavement Thickness Statistics - 50S293B - Drop Height 2						
Subsection	Station	Subgrade Modulus	Effective SN				
1	-20	23968	9.00				
	-10	27014	9.55				
	0		9.55				
	25	25790	8.95				
	50	25636	8.90				
	75	24770	8.60				
	100	26542	8.70				
	125	23414	8.60				
	150	22232	8.85				
	175	25195	8.95				
	200	20846	8.80				
Subsection 1	Overall Mean	24830	8.95				
	Standard Deviation	2077	0.33				
	Coeff of Variation	8.36%	3.65%				

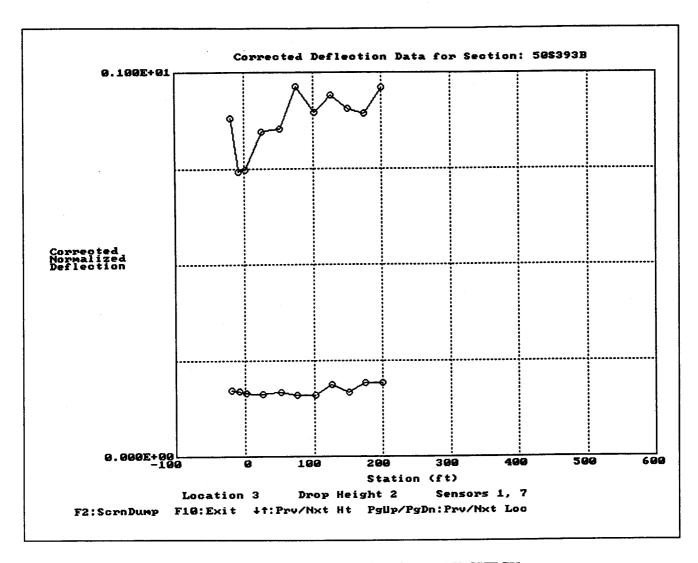


Figure D-9. Deflection Profiles from FWDCHECK (Test Date and Time October 07, 1993 @ 1347)

Table D-7. Subgrade Modulus and Structural Number from FWDCHECK (Test Date and Time October 07, 1993 @ 1347)

Flexible Pavement Thickness Statistics - 50S393B - Drop Height 2						
Subsection	Station	Subgrade Modulus	Effective SN			
1	-20	24279	8.65			
	-10	24403	9.40			
	0	25129	9.35			
	25	25587	8.80			
	50	24875	8.80			
	75	24500	8.30			
	100	25911	8.55			
	125	22382	8.50			
	150	24527	8.55			
	175	21667	8.75			
	200	21772	8.45			
Subsection 1	Overall Mean	24094	8.74			
	Standard Deviation	1480	0.35			
	Coeff of Variation	6.14%	4.02%			

Table D-8. Surface Elevation Measurements

LTPP Seasonal Monitoring Study		State Code	[50]
Surface Elevation	on Measurements	Test Section Number	[1002]
Survey Date	October 07	, 1993	
Surveyed By	MZ & PZ		
Surface Type	A/C		
Benchmark	Observation	n Piezometer - 1.000 meters - a	ssumed

STAT	ION	PE m offset 0.15m	OWP m offset 0.91m	ML m offset 1.83m	TWP m offset 2.59m	ILF m - 4 offset 3.51m
0-25	3+00	1.469	1.485	1.512	1.512	1.524
0-15	3+25	1.472	1.485	1.509	1.512	1.527
0-10	3+50	1.469	1.482	1.509	1.512	1.527
0+00	3+75	1.463	1.475	1.509	1.506	1.524
0+25	2 4+00	1.448	1.466	1.491	1.494	1.512
0+50	% 4+25	1.430	1.448	1.472	1.482	1.500
0+75	** 4+50	1.408	1.421	1.454	1.451	1.475
1+00	4+75	1.393	1.405	1.436	1.439	1.457
1+25	5+00	1.366	1.375	1.405	1.408	1.430
1+50	5+10	1.338	1.354	1.384	1.384	1.408
1+75	5+20	1.317	1.335	1.366	1.366	1.418
2+00	5+30	1.290	1.335	1.338	1.341	1.360

PE	Pavement Edge	
OWP	Outer Wheel Path	
ML	Mid Lane	
IWP	Inner Wheel Path	
ILE	Inner Lane Edge	

APPENDIX E

Photographs

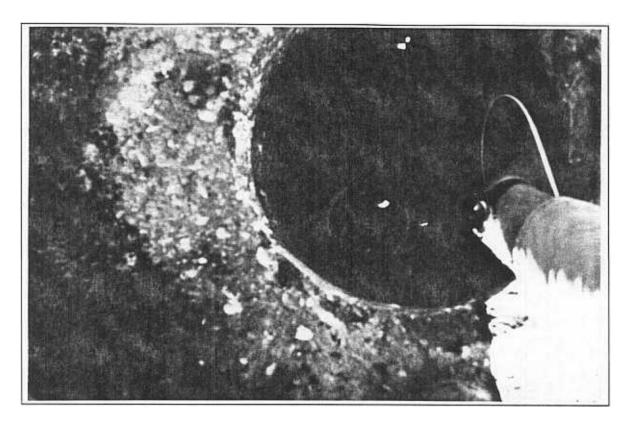


Figure E-1. Instrumentation Hole

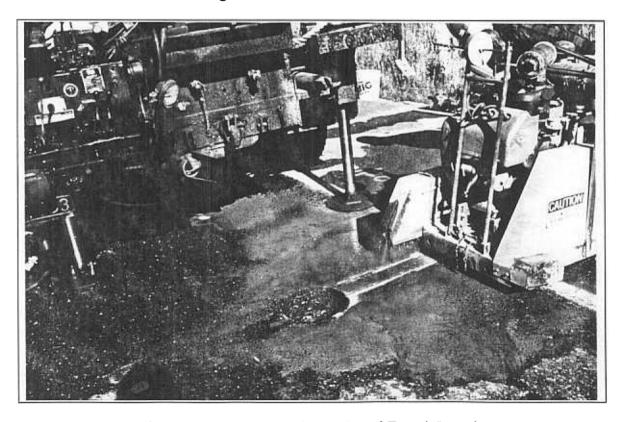


Figure E-2. Instrumentation Hole and Trench Location

E-1

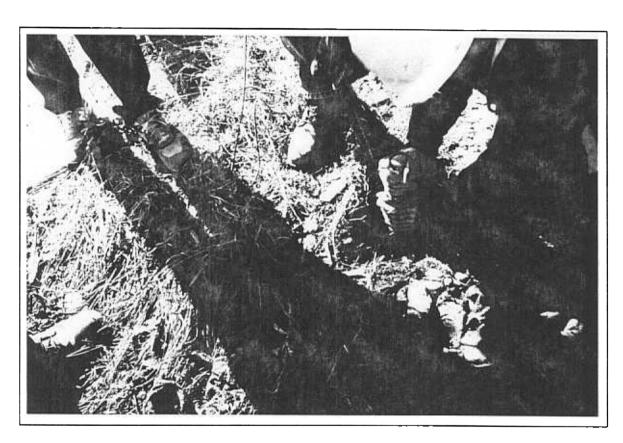


Figure E-3. Observation Well

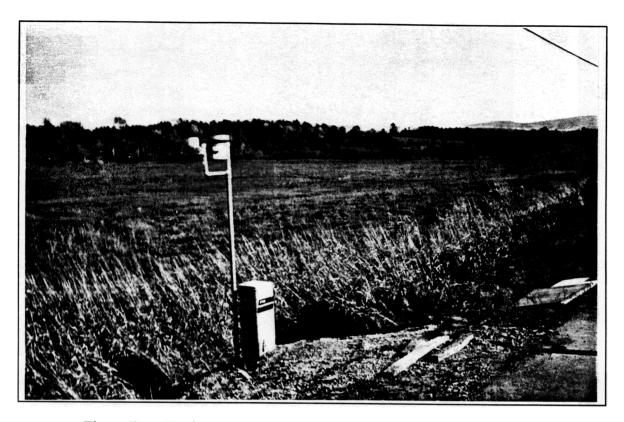


Figure E-4. Equipment Cabinet, Air Temperature Probe, and Rain Gage

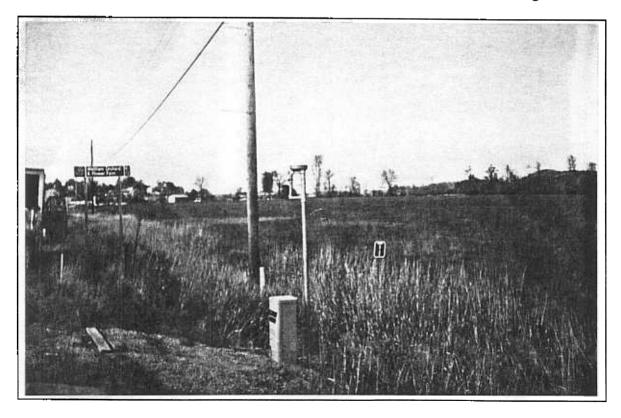


Figure E-5. Equipment Cabinet, Air Temperature Probe, and Rain Gage

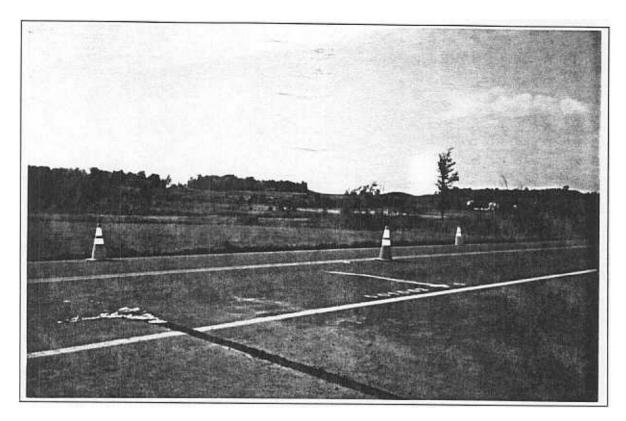


Figure E-6. Patch Area, the Day After Installation

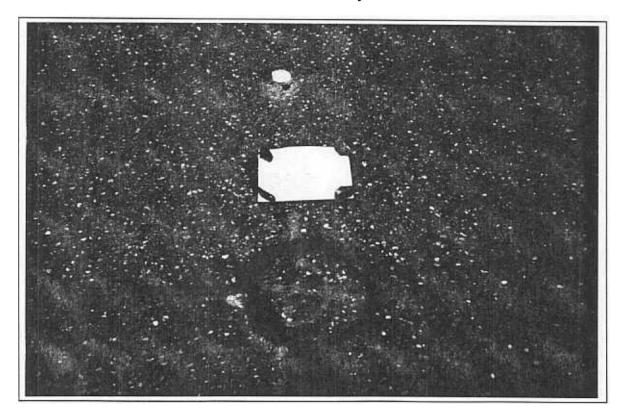


Figure E-7. Patch Area, Two and a Half Months After Installation

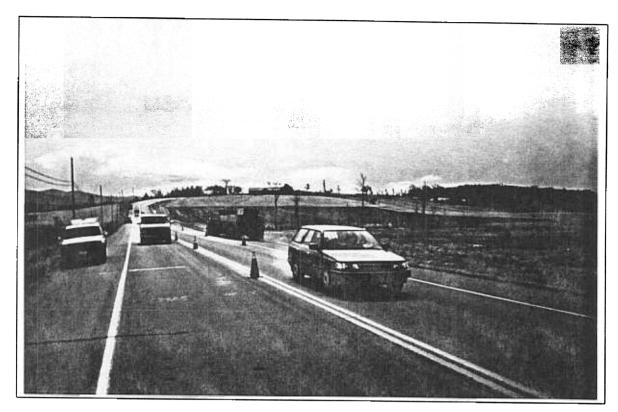


Figure E-8. Traffic Control and FWD Testing, Site 501002, April 3, 1991 (State Exp.)

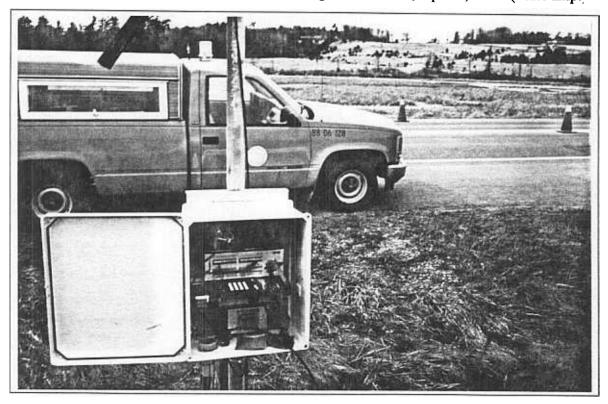


Figure E-9. Equipment Cabinet, Site 501002, April 3, 1991 (State Experiment)

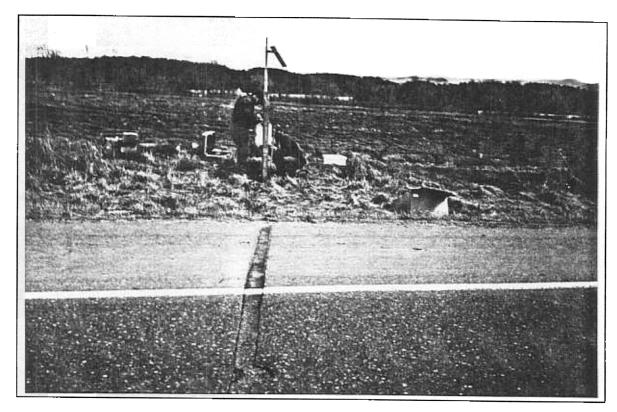


Figure E-10. Trench and Equipment Cabinet, Site 501002, April 3, 1991 (State Exp.)

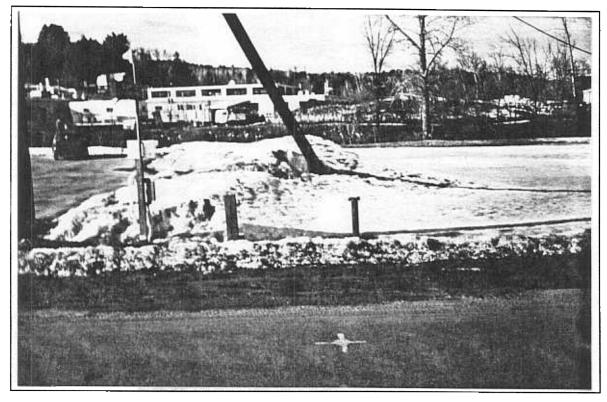


Figure E-11. Equipment Cabinet Location, Town Highway 27, April 4, 1991 (State Exp.)